LOCAL TECHNICAL ASSISTANCE PROGRAM

2024 AASHTO BIKE GUIDE

March 2025

Pennsylvania Department of Transportation

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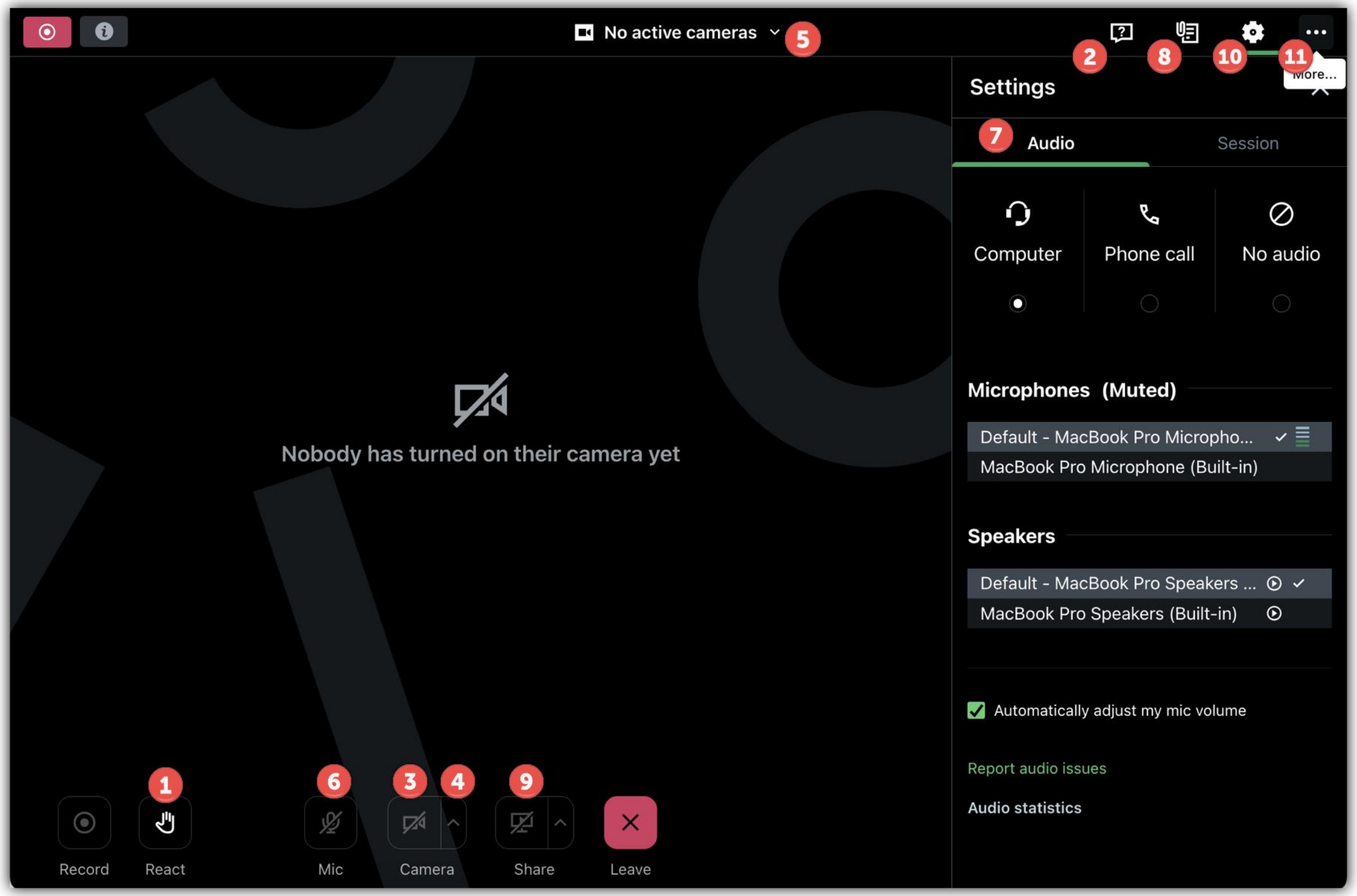

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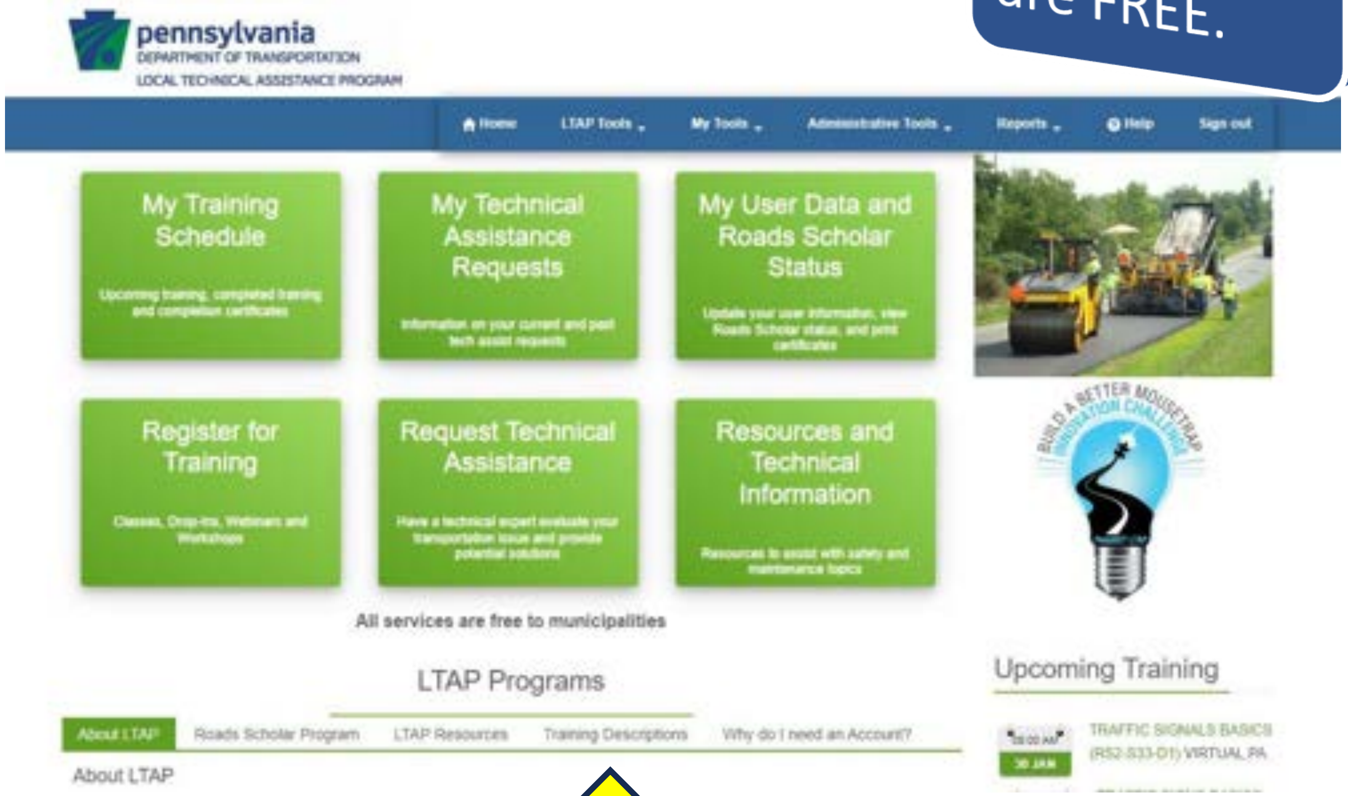

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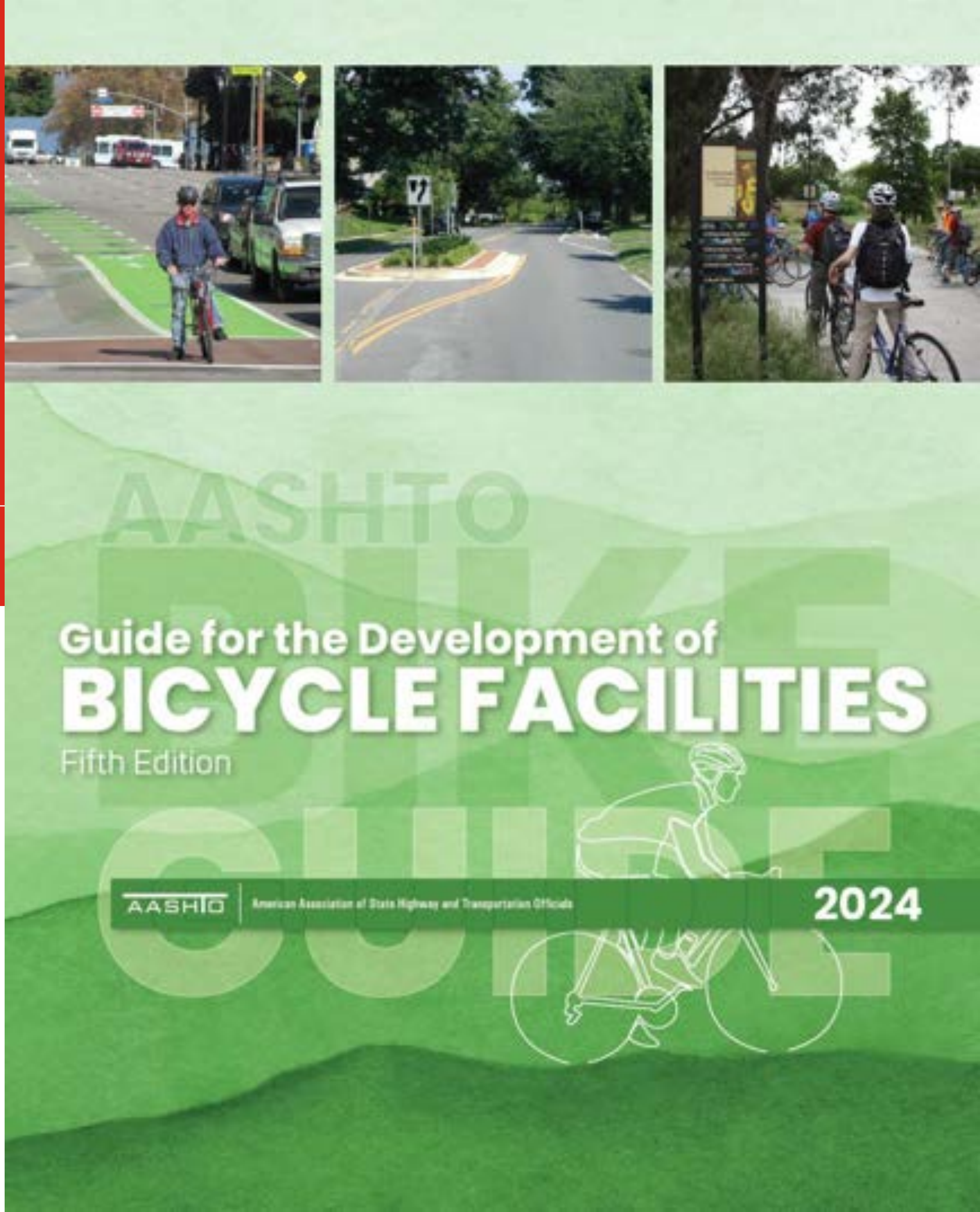
2024 AASHTO Bike Guide 5th Edition

PennDOT Local Technical Assistance Program
Thursday, March 6, 2025

Katy Sawyer, PE
Principal Engineer

TOOLE DESIGN



Chapter 1 – Introduction

1.1 Design Imperative for Bicycle Facilities

1.2 Purpose

1.3 Design Flexibility

1.4 Use of Values in the Guide

1.5 Scope

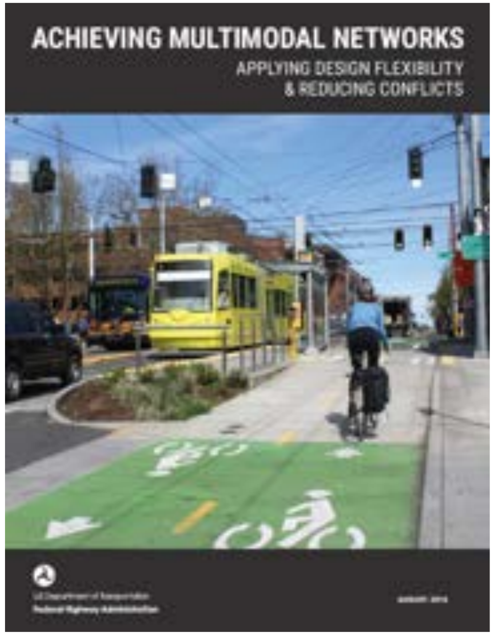
1.6 Relationship to other Design Guides and Manuals

1.7 Structure of this Guide

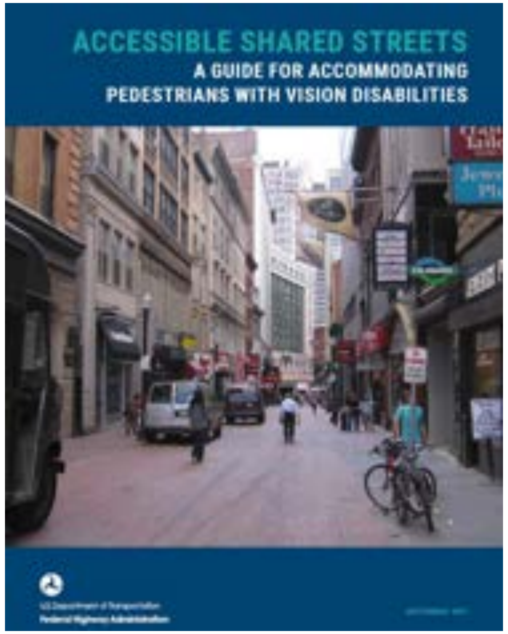
1.8 Definitions

Section 1.6 - Relationship to Other Manuals

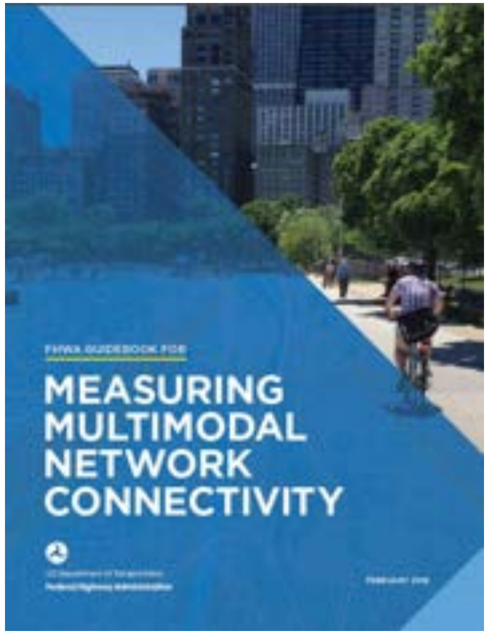
FHWA Separated Bike Lane Planning and Design Guide
May 2015



FHWA Achieving Multimodal Networks
August 2016



FHWA Accessible Shared Streets
September 2017



FHWA Measuring Multimodal Network Connectivity
February 2018

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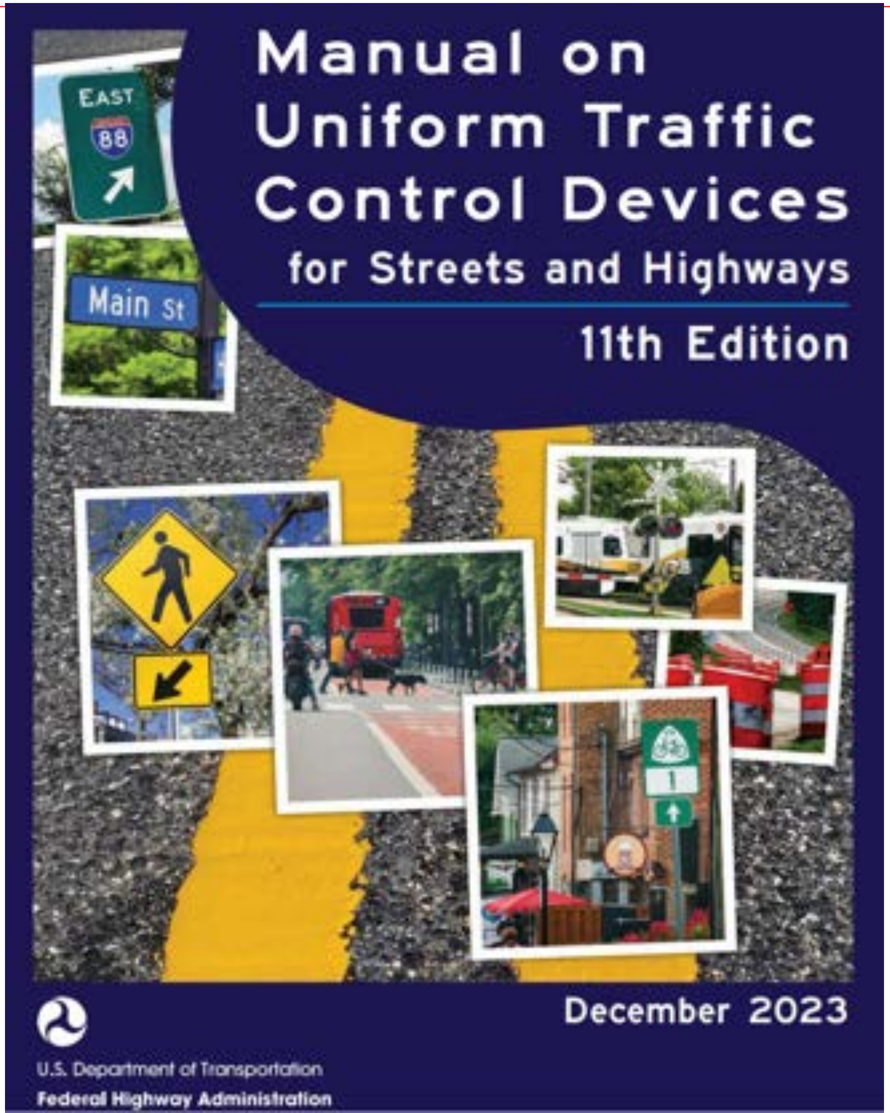
1.6.1. Manual on Uniform Traffic Control Devices for Streets and Highways (MUTCD)

MUTCD defines design and application of traffic control devices (TCDs).

2024 Bike Guide conforms to 2023 MUTCD

Includes some TCDs that require experimental approval by FHWA (located at the end of their respective section)

AASHTO expands upon the application of TCDs



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Chapter 2 - Bicycle Operation and Safety

2.1. Introduction

2.2 Safety of Bikeways and Shared Lanes

2.3. Bicyclist Design User Profiles

2.4. Bicyclist Safety and Performance Characteristics

2.5. Design Vehicle and Bicyclist Operating Criteria

2.6. Operating Principles for Bicyclists

2.7. Guiding Principles for Bicyclist Safety

2.2.1. Relationship between Perceived Comfort and Substantive Safety

Research has found a significant relationship between

- how safe and comfortable people feel bicycling,
- whether and how often they bicycle,
- preferences for facility types, and the provision of those facilities.

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2.2.1. Relationship between Perceived Comfort and Substantive Safety

Crashes and near-crash experiences influence perceived bicycling safety and comfort

(Lee et al., 2015; Sanders, 2015; Aldred & Crossweller, 2015)

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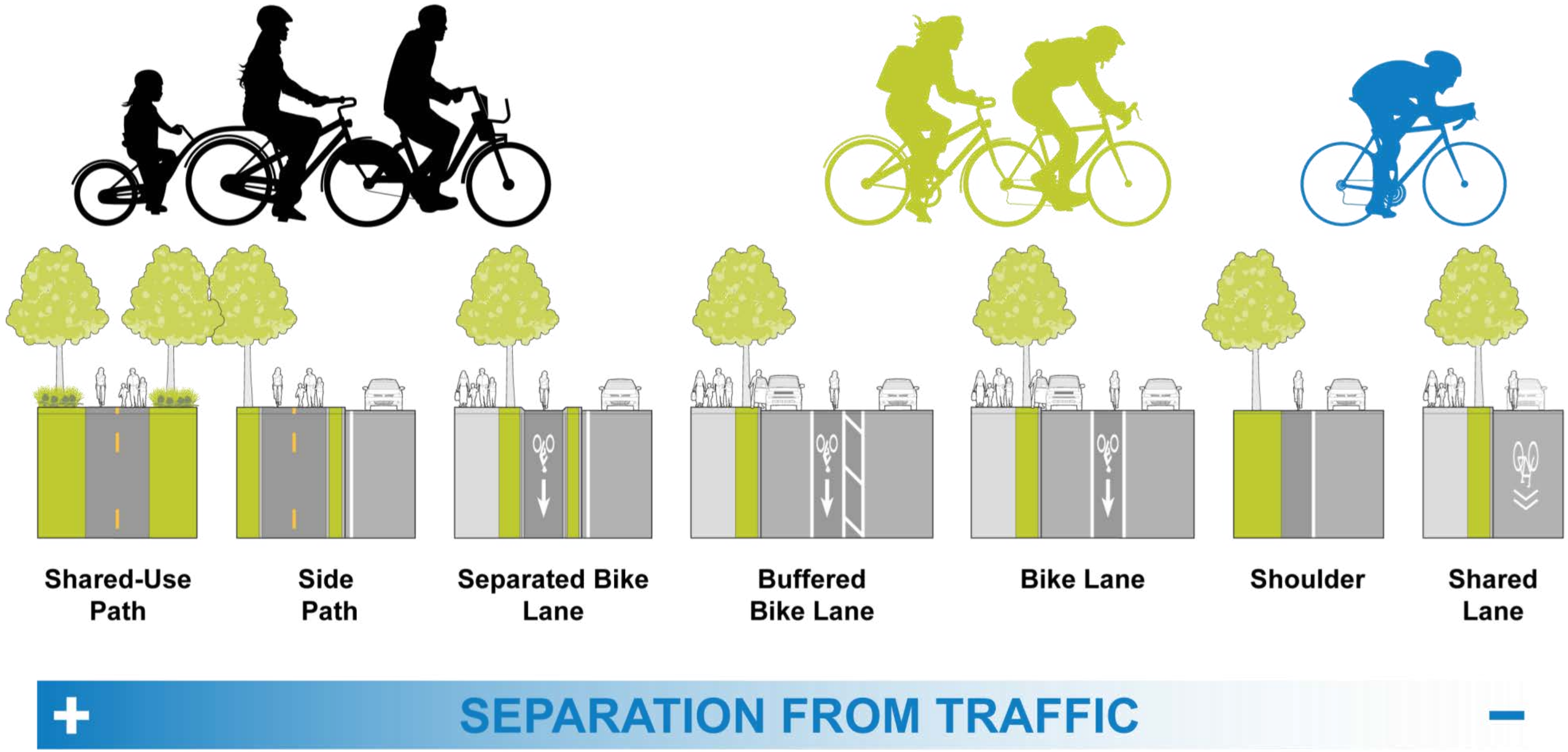
Comfort Increases with Separation



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2.3. Bicyclist Design User Profiles

Comfort Typology of Bicyclists

Design User Profile	Non-Bicyclist	Interested but Concerned	Somewhat Confident	Highly Confident
Bicycling Preferences	Uncomfortable bicycling in any condition, have no interest in bicycling, or are physically unable to bicycle.	Often not comfortable with bike lanes; may bike on sidewalks even if bike lanes are provided. Prefer off-street or separate bicycle facilities or quiet or traffic-calmed residential roads. May not bike at all if bicycle facilities do not meet needs for perceived comfort.	Generally prefer more separated facilities, but comfortable riding in bicycle lanes or on paved shoulders if need be.	Comfortable riding with traffic; will use roads without bike lanes.
Percent of Population	31–37%	51–56%	5–9%	4–7%

Figure 2-2: Comfort Typology of Bicyclists (See Chapter 2 References: Dill and McNeill, 2016)

Chapter 3: Bicycle Planning

3.1 Introduction

3.2 Bicycle Planning Principles

3.3 Primary Considerations for Bicycle Planning

3.4 Planning For Desired Outcomes

3.5 Deciding Where Improvements Are Needed

3.6 Integrating Bicycle Facilities with Transit (First- and Last-Mile Connections)

3.7 Bike Parking and End of Trip Support

3.8 Types of Transportation Planning Processes

3.9 Technical Analysis Tools That Support Bicycle Planning

3.10 Public Input

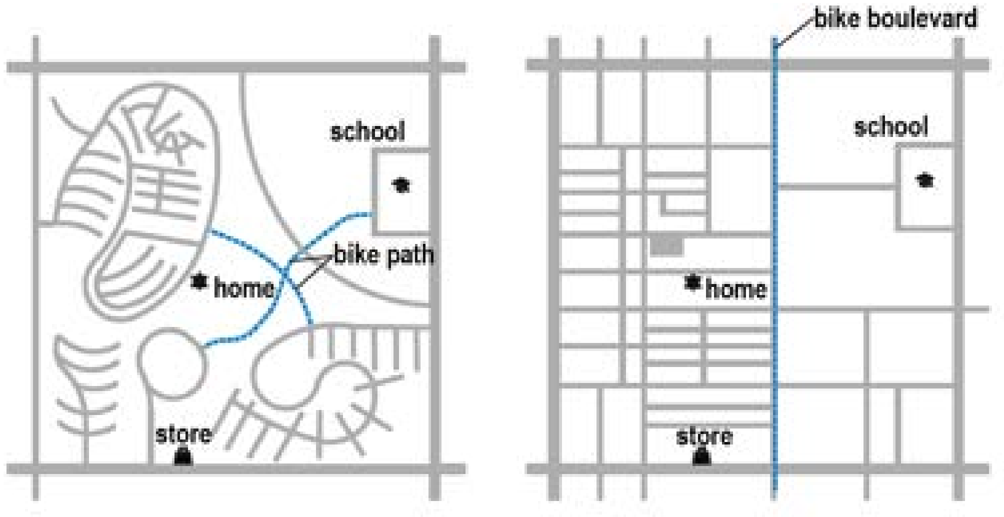
Bicycle Planning Principles

3.2.1. Safety – reduce frequency and severity of crashes by separating bicyclists from higher speed and volumes of motorists

3.2.2. Comfort – do not deter use due to safety concerns

3.2.3. Connectivity – direct, complete and continuous

3.2.4. Legibility – easy to recognize and intuitive to use



Improved Bicycle Connectivity within poorly connected road network

Improved Bicycle Connectivity within well connected road network

Figure 3-1: Examples of Contrasting Connectivity

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3.9.2. Quality of Service and Bicycle Level of Service Tools

3.9.2.2 Level of Traffic Stress

objective and quantitative method of classifying road segments and bikeway networks based on how comfortable bicyclists

Figure 3-3: Example of Bicycle Master Plan Recommendations Map[24]

Table 3-4: Levels of Traffic Stress[29]

Levels of Traffic Stress (LTS)	
LTS 1	Presenting little traffic stress and demanding little attention from cyclists, and attractive enough for a relaxing bike ride. Suitable for almost all cyclists, including children trained to safely cross intersections. On links, cyclists are either physically separated from traffic, or are in an exclusive bikeway next to a slow traffic stream with no more than one lane per direction, or are on a shared road where they interact with only occasional motor vehicles (as opposed to a stream of traffic) with a low speed differential. Where cyclists ride alongside a parking lane, they have ample operating space outside the zone into which car doors are opened. Intersections are easy to approach and cross.
LTS 2	Presenting little traffic stress and therefore suitable to most adult cyclists but demanding more attention than might be expected from children. On links, cyclists are either physically separated from traffic, or are in an exclusive bicycling zone next to a well-confined traffic stream with adequate clearance from a parking lane, or are on a shared road where they interact with only occasional motor vehicles (as opposed to a stream of traffic) with a low speed differential. Where a bike lane lies between a through lane and a right-turn lane, it is configured to give cyclists unambiguous priority where motor vehicles cross the bike lane and to keep speeds in the right-turn lane comparable to bicycling speeds. Crossings are not difficult for most adults.
LTS 3	More traffic stress than LTS 2, yet markedly less than the stress of integrating with multilane traffic, and therefore welcome to many people currently riding bikes in American cities. Offering cyclists either an exclusive bikeway next to moderate-speed traffic or shared lanes on streets that are not multilane and have moderately low speed. Crossing may be longer or across higher-speed roads than allowed by LTS 2, but still considered acceptably safe to most adult bicyclists.
LTS 4	A level of stress beyond LTS 3. Bicyclist mix with motor vehicle traffic. Generally uncomfortable for most adults.

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Chapter 4 - Guidance for Choosing a Bikeway Type

4.1 Introduction

4.2 Project Performance Goals and Objectives

4.3 Selecting the Preferred Bikeway Type

4.4 Strategies to Achieve the Preferred (or Next Best) Design

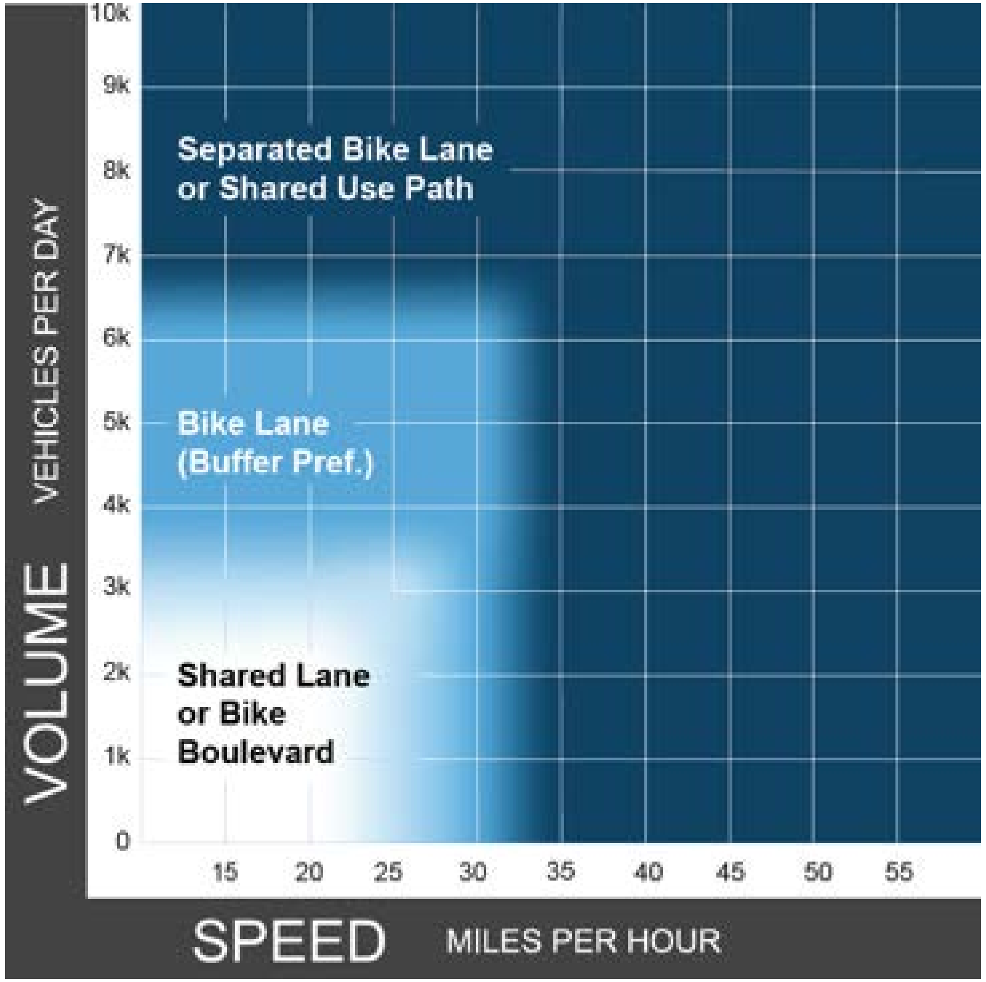
4.5 Evaluating Design Alternatives and Trade-offs to Select a Bikeway

Section 4.3.1 – Streets in Urban, Suburban and Rural Town Contexts

Identifies the **preferred** bikeway type assuming:

<u>Design User</u> = Interested but Concerned bicyclist

<u>Analysis</u> = Level of Traffic Stress



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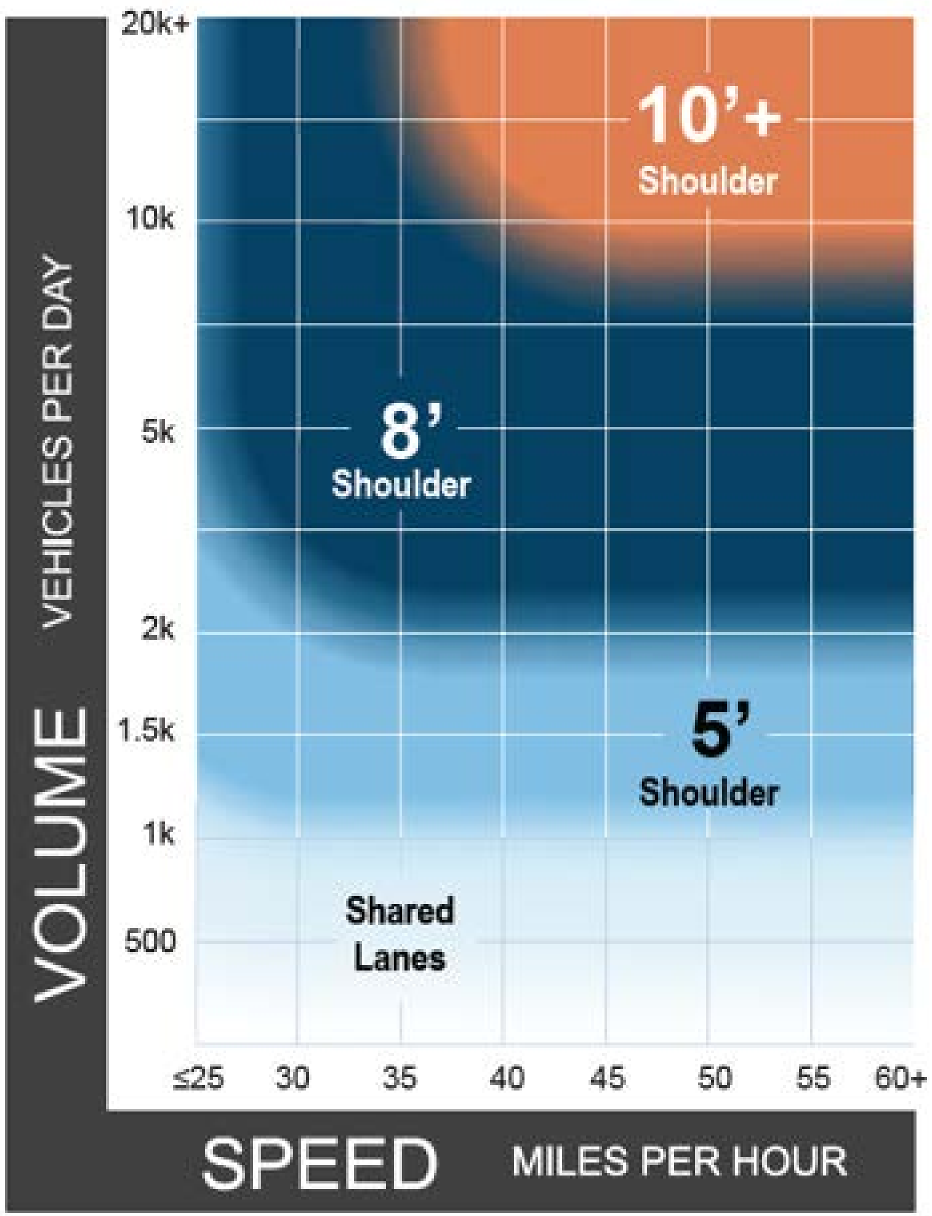
Section 4.3.2 – Rural Roadways

Identifies the **preferred** shoulder width assuming:

<u>Design User</u> = Confident bicyclist

<u>Analysis</u> = Bicycle LOS

Figure 4-2: Preferred Paved Shoulder Widths for Rural Roadways to Accommodate Highly Confident or Somewhat Confident Bicyclists



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4.4.2. Example Strategies for Constrained Rights-of-Way

4.4.2.1 Traffic Analysis Approach

4.4.2.2 Narrowing Travel Lanes

4.4.2.3 Removing Travel Lanes

4.4.2.4 Reorganizing Street Space

4.4.2.5 Making Changes to On-Street Parking

4.4.2.6 Reducing Bikeway Widths

4.4.2.7 Reducing Motor Vehicle Traffic Volumes and Speeds

4.5.2. Example of Trade-off Considerations Between Common Bikeway Types

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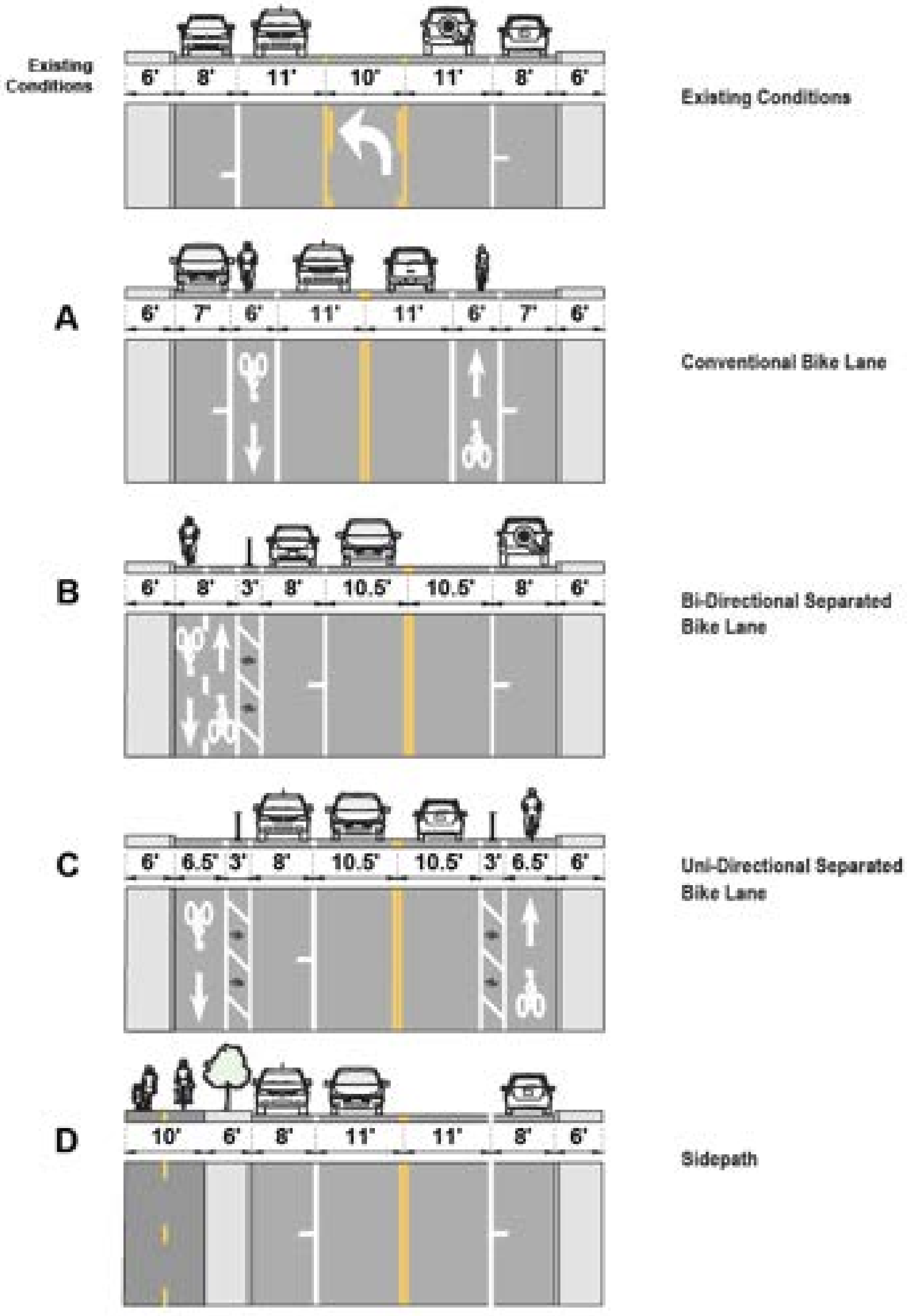


Figure 4-3: Common Bikeway Options within a 48-ft Cross Section

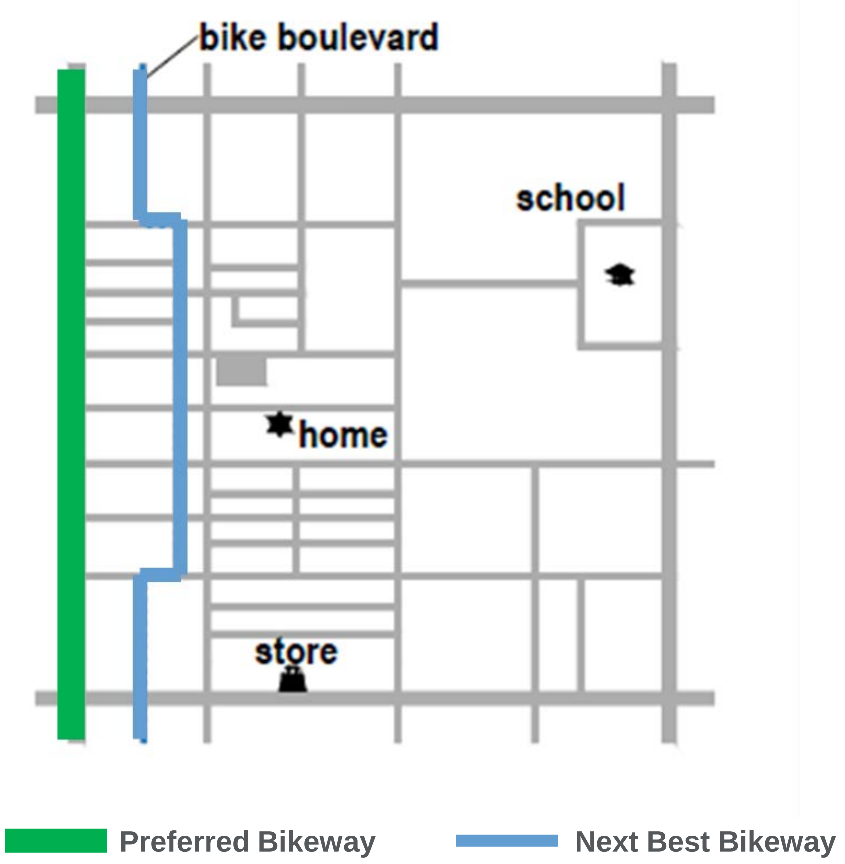
4.5.3. Selecting the Next Best Facility When the Preferred Bikeway Is Not Feasible

Alternative Route

If no other design improvements are feasible, it is necessary to consider alternative parallel routes.

Research indicates that for an alternative low-stress route to be viable, **the increase in trip length should be less than 30 percent.**

Broach, J., Dill, J., and J., Gliebe. Where Do Cyclists Ride? A Route Choice Model Developed with Revealed Preference GPS Data



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Chapter 5 – Elements of Design

5.1 Introduction

5.2 Design User

5.3 Design Speed

5.4 Understanding Assignment of Right of Way

5.5 Sight Distance

5.6 Surface and Geometric Design Elements

5.7 Characteristics of Intersections

5.8 Intersection Design Objectives

5.9 Evaluating Bicycle and Pedestrian Roadway Crossings

5.10 Geometric Design Treatments to Improve Intersection Safety

5.11 Warning and Regulatory Traffic Control Devices

5.12 Pavement Markings

5.13 Bicycle Travel Near Rail Lines

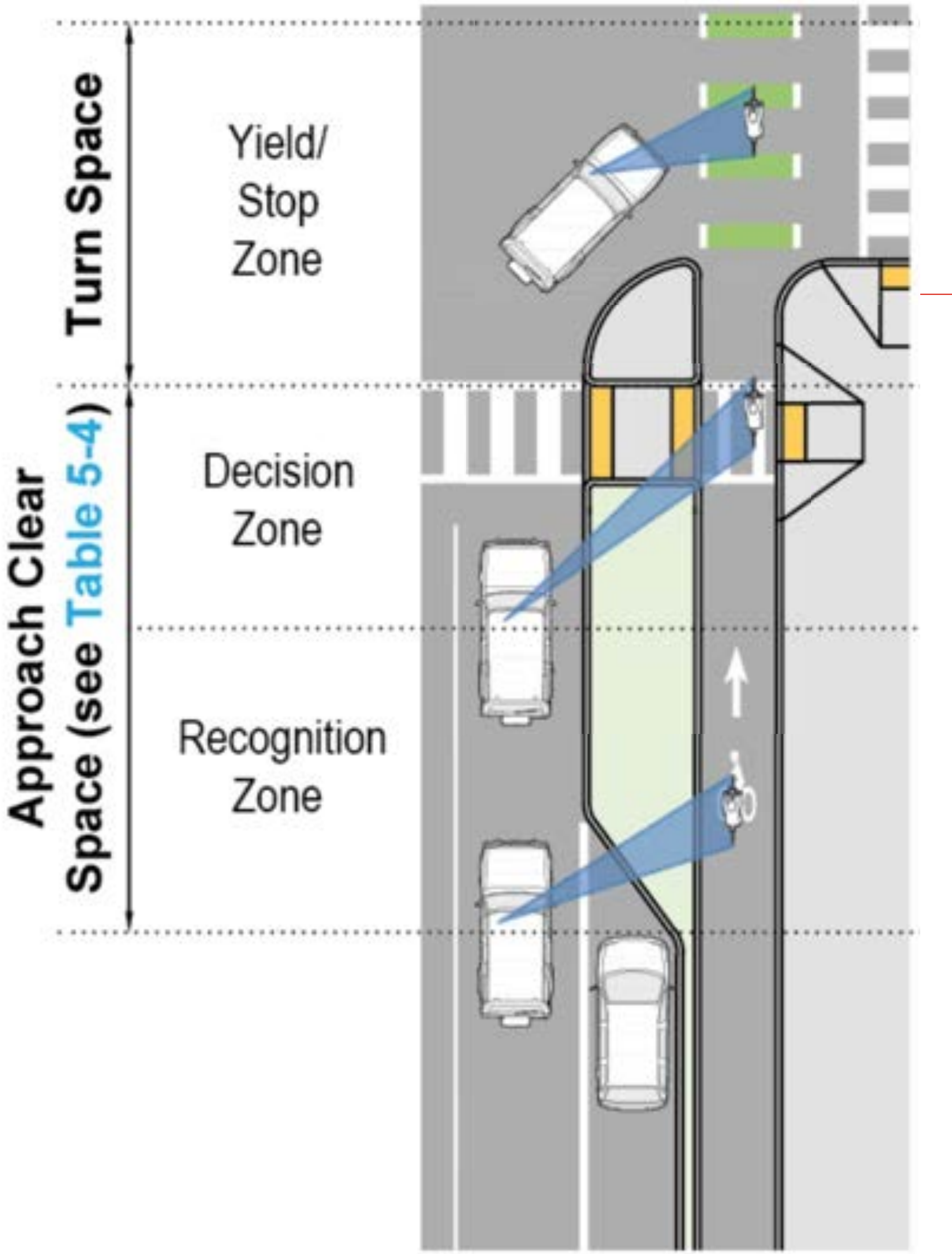
5.14 Other Design Features

Section 5.4 – Understanding Assignment of Right of Way

All street users need opportunity for Mutual Identification because:

- Motorists & bicyclists must yield to pedestrians in crosswalks
- Pedestrians cannot suddenly leave the curb if vehicles too close to stop
- Motorists must exercise due care to avoid colliding with bicyclists/peds

The approach to a conflict point is composed of three zones.



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5.5.2. Stopping Sight Distance

Tables provided for:

- Unexpected Conflict, 2.5 second PRT
- Expected Conflict, 1.5 second PRT

Table 5-2: Minimum Bicyclist Stopping Sight Distance vs. Grades for Various Design Speeds—2.5-Second Reaction Time

Stopping Sight Distance (ft) Based on Speed and Grade for a 2.5-Second Perception-Reaction Time											
Speed (mph)	Grade (Positive indicates ascending)										
	–10%	–8%	–6%	–4%	–2%	0	2%	4%	6%	8%	10%
10				65	61	58	55	53	52	51	50
11				74	69	66	63	61	59	57	56
12				84	78	74	71	68	66	64	62
15			130	118	109	102	97	93	89	86	84
18	246	201	174	156	143	134	126	120	115	111	108
20	296	240	207	185	169	157	148	140	134	129	
25	440	353	300	266	241	222	208	196	187		
30	611	486	411	361	325	298	277	260			

Note: Calculations are assumed under wet conditions.

Table 5-3: Minimum Bicyclist Stopping Sight Distance vs. Grades for Various Design Speeds—1.5-Second Reaction Time

Stopping Sight Distance (ft) Based on Speed and Grade for a 1.5-Second Perception-Reaction Time											
Speed (mph)	Grade (Positive indicates ascending)										
	–10%	–8%	–6%	–4%	–2%	0	2%	4%	6%	8%	10%
10%				50	46	43	41	39	37	36	35
11				58	53	49	47	44	43	41	40
12				66	61	56	53	50	48	46	45
15			108	96	87	80	75	71	67	64	62
18	220	175	148	130	117	107	100	94	89	85	81
20	267	211	178	155	139	128	118	111	105	100	
25	403	316	264	229	204	185	171	159	150		
30	567	442	367	317	281	254	233	216			

Note: Calculations are assumed under wet conditions.

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5.5.4.1 Sight Distance and Approach Clear Space for Bikeways at Roadway Intersections

- **Turning Motorist Yields to (or Stops for) Through Bicyclists:** When a through moving bicyclist that arrives or will arrive at the crossing prior to a turning motorist, the motorist must stop or yield.
- **Through Bicyclist Yields to (or Stops for) Turning Motorist:** When a turning motorist arrives or will arrive at the crossing prior to a through moving bicyclist, the bicyclist must stop or yield.
- **User with Right-of-Way Yields to (or Stops for) Another User:** Sometimes the user with the right-of-way will instead yield the right-of-way.
- **APPROACH CLEAR SPACE ALLOWS THIS TO FUNCTION!**

5.5.4.1.1 Case S – Right-Turning Motorist Across Separated Bike Lane or Side Path

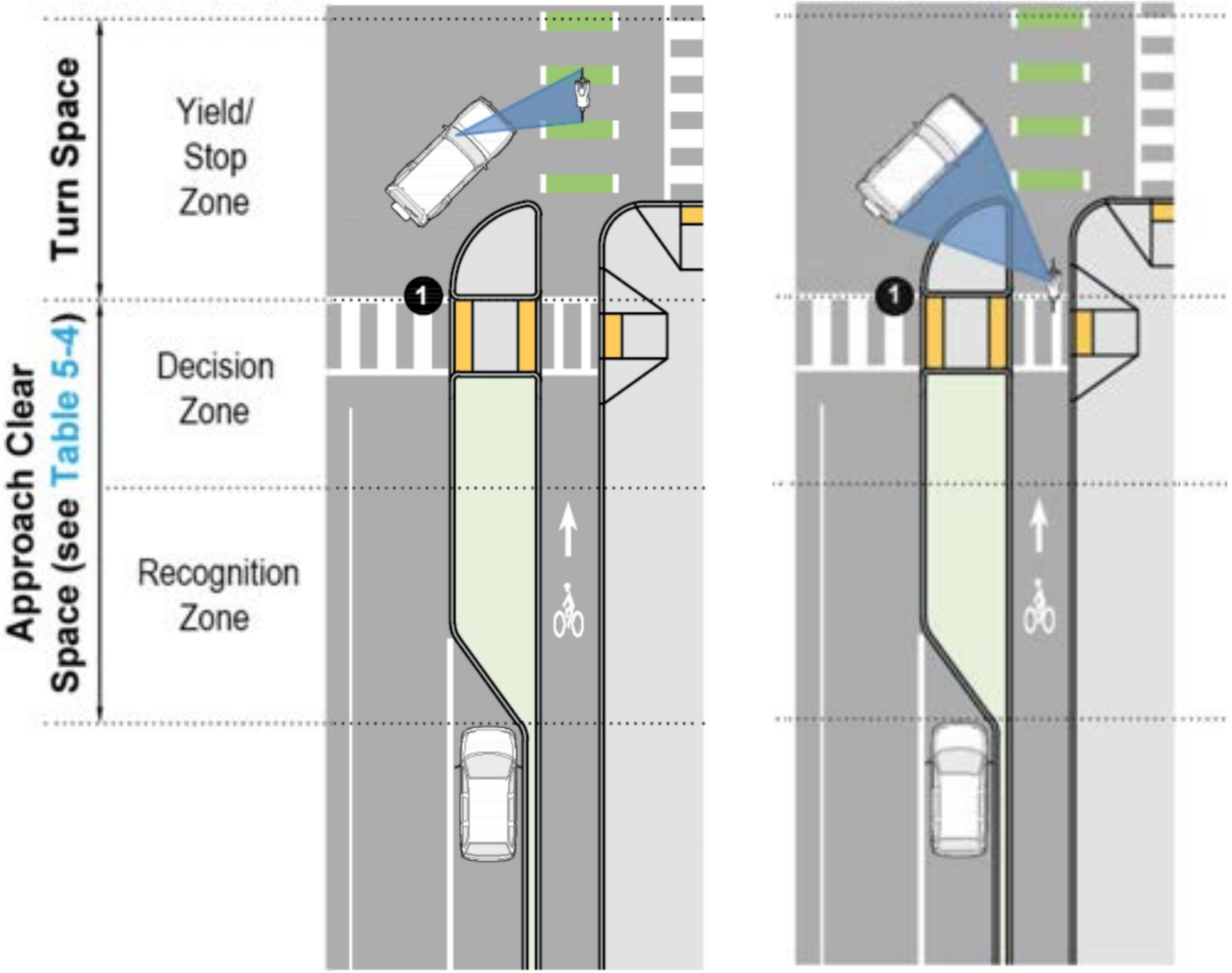
Table 5-4: Recommended Intersection Approach Clear Space by Vehicular Turning Design Speed

Effective Vehicle Turning Radius	Vehicular Turning Speed	Recommended Approach Clear Space
<18 ft	<10 mph[a]	20 ft
18 ft	10 mph	40 ft
25 ft	15 mph	50 ft
30 ft	20 mph	60 ft
>30 ft	25 mph	70 ft

[a] Most low-volume driveways and alleys

legend

line of sight



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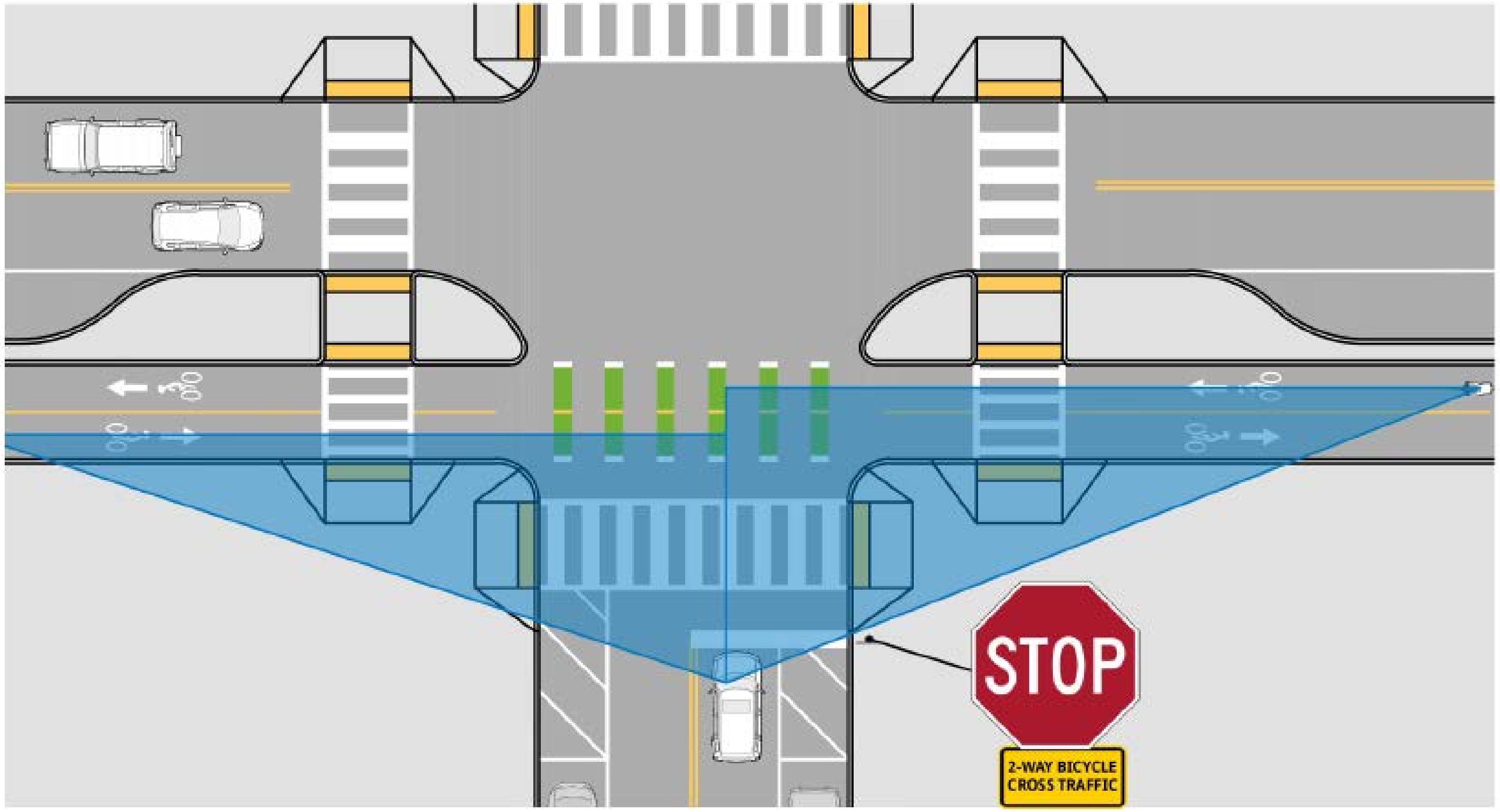
5.5.4.1.3 Case U1 – Through Motorist Crossing of a Separated Bike Lane or Shared Use Path



at a minimum the **provision of stopping sight distance for bicyclists** (Section 5.5.2) **should be provided** to allow a bicyclist to slow or stop if a vehicle encroaches into the separated bike lane or side path

Figure 5-3: Intersection Sight Distance: Case U1

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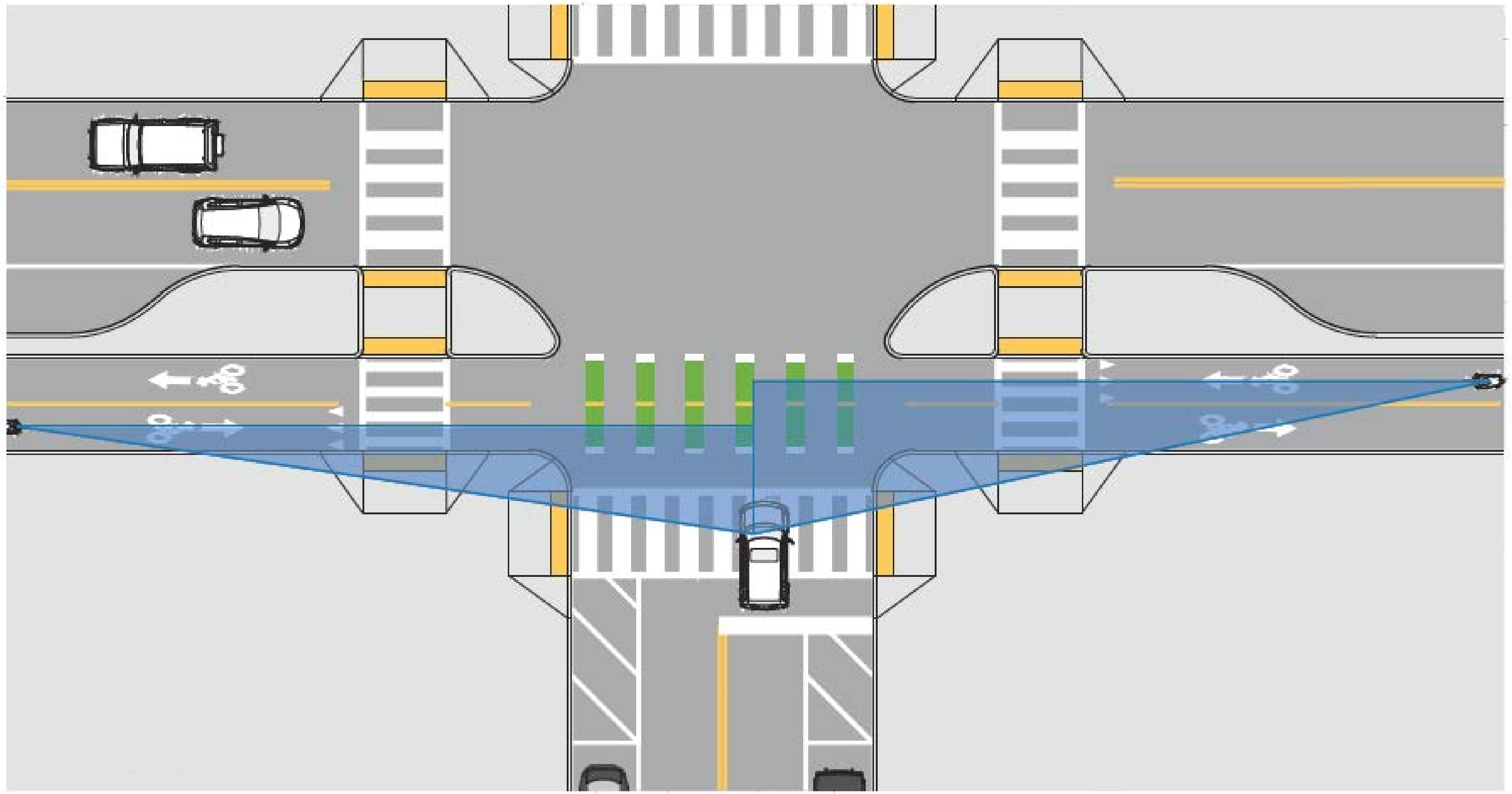
7.9.5 Case U1 – Multistep Variant



Chapter 7 sight distance

- Driver looks for pedestrians, then moves forward
- Driver looks for bicyclists, then moves forward
- Driver looks for other motorists, then proceeds

5.5.4.1.3.3 U3 – Mid-Block Shared Use Path Crossing of an Uncontrolled Roadway

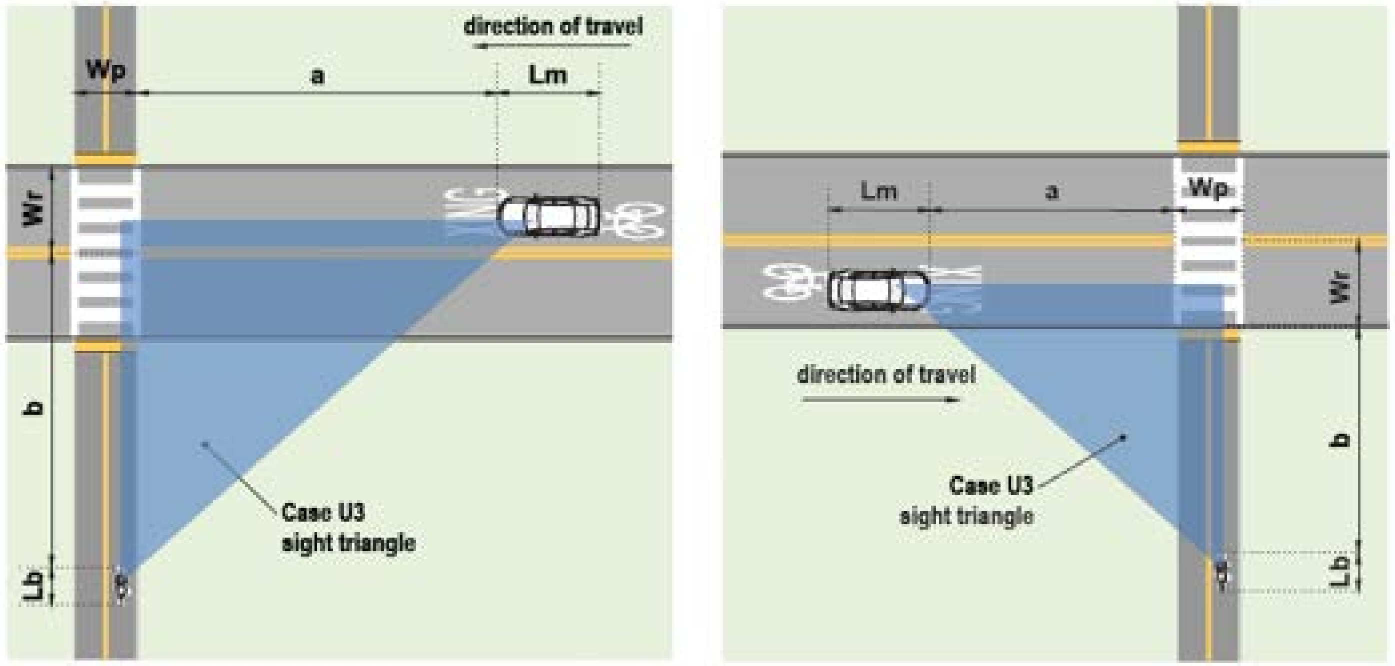


Figure 5-5: Sight Triangle for Uncontrolled Mid-Block Path Crossing of an Uncontrolled Roadway: Case U3

Table 5-8: Length of Path and Roadway Sight Triangle for Uncontrolled Crossings: Case U3

Length of Path and Roadway Sight Triangle (ft) - Case U3						
Bike Speed (mph)	Roadway Speed (mph)					
	15	20	25	30	35	40
10	96 / 58	128 / 59	160 / 63	192 / 68	224 / 74	255 / 81
11	97 / 64	129 / 65	162 / 69	194 / 75	226 / 82	258 / 89
12	98 / 70	131 / 70	164 / 75	197 / 82	230 / 89	262 / 97
15	105 / 87	140 / 88	174 / 94	209 / 102	244 / 111	279 / 122
18	112 / 105	150 / 106	187 / 113	225 / 122	262 / 134	300 / 146
20	118 / 116	157 / 117	197 / 125	236 / 136	275 / 149	315 / 162
25	133 / 145	178 / 147	222 / 156	266 / 170	311 / 186	355 / 203
30	149 / 174	199 / 176	249 / 188	298 / 204	348 / 223	398 / 244

a / b: a = sight distance (ft) along roadway; b = sight distance (ft) along path

Assumptions:
- Bicycle reaction time = 1.5 seconds
- Width of path = 10 ft to 11 ft
- Width of road lane = 11 ft to 12 ft
- Length of bicycle = 6 ft
- Length of motor vehicle = 18 ft
- Grade = –2 percent to +2 percent

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5.5.4.3 Sight Distance at Horizontal Curves

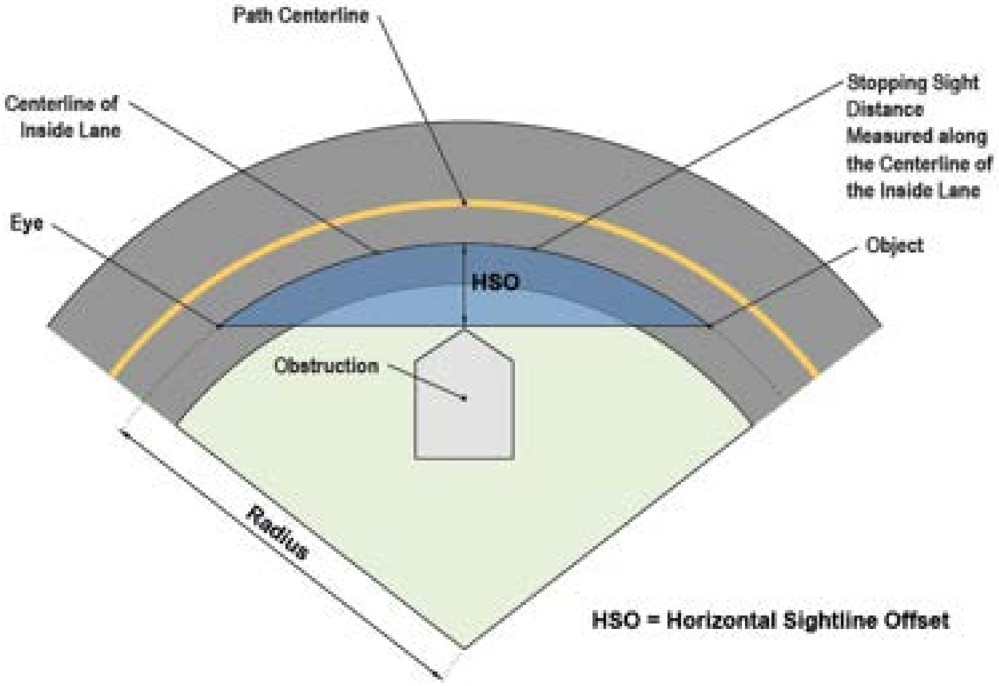

Figure 5-10: Diagram Illustrating Components for Determining Horizontal Sightline Offset

Table 5-11: Horizontal Sightline Offset Look-Up

Minimum Lateral Clearance (Horizontal Sightline Offset or HSO) for Horizontal Curves (ft)														
R (ft)	S = Stopping Sight Distance (ft)													
	40	60	80	100	120	140	160	180	200	220	240	260	280	300
25	7.6	15.9												
50	3.9	8.7	15.2	23.0	31.9	41.5								
75	2.7	5.9	10.4	16.1	22.8	30.4	38.8	47.8	57.4	67.2				
100		4.5	7.9	12.2	17.5	23.5	30.3	37.8	46.0	54.6	63.8	73.3	83.0	92.9
125		3.6	6.3	9.9	14.1	19.1	24.7	31.0	37.9	45.4	53.3	61.7	70.6	79.7
150		3.0	5.3	8.3	11.8	16.0	20.8	26.2	32.1	38.6	45.5	52.9	60.7	69.0
175		2.6	4.6	7.1	10.2	13.8	18.0	22.6	27.8	33.5	39.6	46.1	53.1	60.5
200			4.0	6.2	8.9	12.1	15.8	19.9	24.5	29.5	34.9	40.8	47.0	53.7
225			3.5	5.5	8.0	10.8	14.1	17.8	21.9	26.4	31.3	36.5	42.2	48.2
250			3.2	5.0	7.2	9.7	12.7	16.0	19.7	23.8	28.3	33.1	38.2	43.7
275			2.9	4.5	6.5	8.9	11.6	14.6	18.0	21.7	25.8	30.2	34.9	39.9
300			2.7	4.2	6.0	8.1	10.6	13.4	16.5	19.9	23.7	27.7	32.1	36.7
350				3.6	5.1	7.0	9.1	11.5	14.2	17.1	20.4	23.9	27.6	31.7
400				3.1	4.5	6.1	8.0	10.1	12.4	15.0	17.9	20.9	24.3	27.8
450				2.8	4.0	5.4	7.1	9.0	11.1	13.4	15.9	18.7	21.6	24.8
500				2.5	3.6	4.9	6.4	8.1	10.0	12.1	14.3	16.8	19.5	22.3
600					3.0	4.1	5.3	6.7	8.3	10.1	12.0	14.0	16.3	18.7
700					2.6	3.5	4.6	5.8	7.1	8.6	10.3	12.0	14.0	16.0
800						3.1	4.0	5.1	6.2	7.6	9.0	10.5	12.2	14.0
900						2.7	3.6	4.5	5.6	6.7	8.0	9.4	10.9	12.5
1000							3.2	4.0	5.0	6.0	7.2	8.4	9.8	11.2

Table 5-12: Horizontal Sightline Offset for Horizontal Curves Equation

Horizontal Sight Line Offset for Horizontal Curves Equation		
$HSO = R\left[1 - \cos\left(\frac{28.65S}{R}\right)\right]$ $S = \frac{R}{28.65}\left[\cos^{-1}\left(\frac{R - HSO}{R}\right)\right]$		
Where:		
S	=	stopping sight distance (ft)
R	=	radius of centerline of lane (ft)
HSO	=	horizontal sightline offset, distance from centerline of lane to obstruction (ft)
Note: Angle is expressed in degrees.		

TOOLE DESIGN

5.8. Intersection Design Objectives

5.8.1. Minimize Exposure to Conflicts

5.8.2. Reduce Speeds at Conflict Points

5.8.3. Communicate Right-of-Way Priority

5.8.4. Providing Adequate Sight Distance

5.8.5. Transitions to Other Facilities

5.8.6. Accommodating Persons with Disabilities

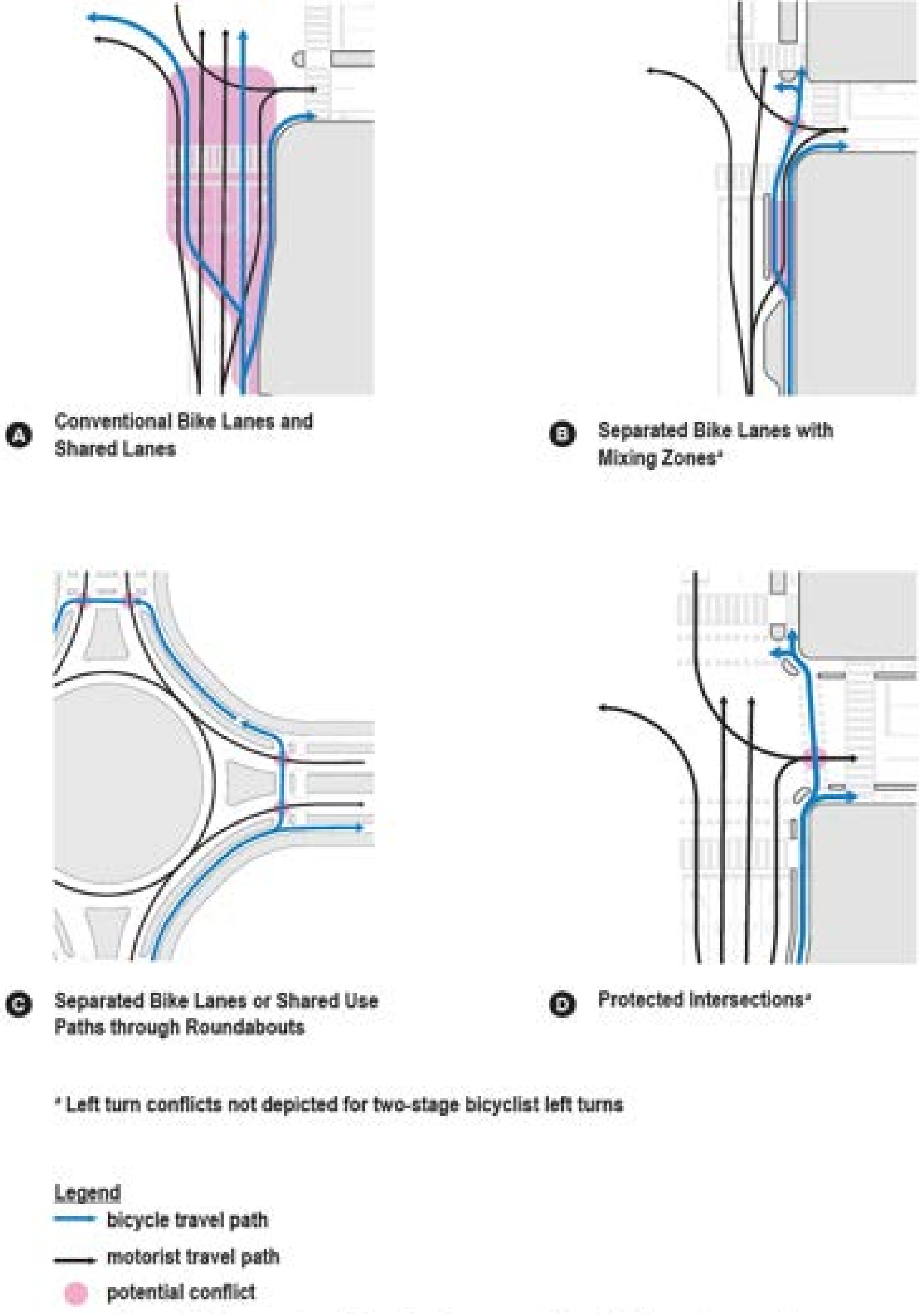

Figure 5-13: Comparison of Bicyclist Exposure to Motor Vehicles at Intersections

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5.9.2. Evaluations of Uncontrolled Roadway Approaches to Bicycle Crossings

5.9.2.1 Factors That Impact Motorist Yielding Rates

5.9.2.2.1 Recommended Crossing Opportunities

Table 5-14: Recommended Minimum Range of Hourly Crossing Opportunities

Major Street Crossings (opportunities per hour)	
Recommended	≥120
Practical Minimum	60 to <120

Motorist Yielding Behavior at Uncontrolled Approaches to Crossings

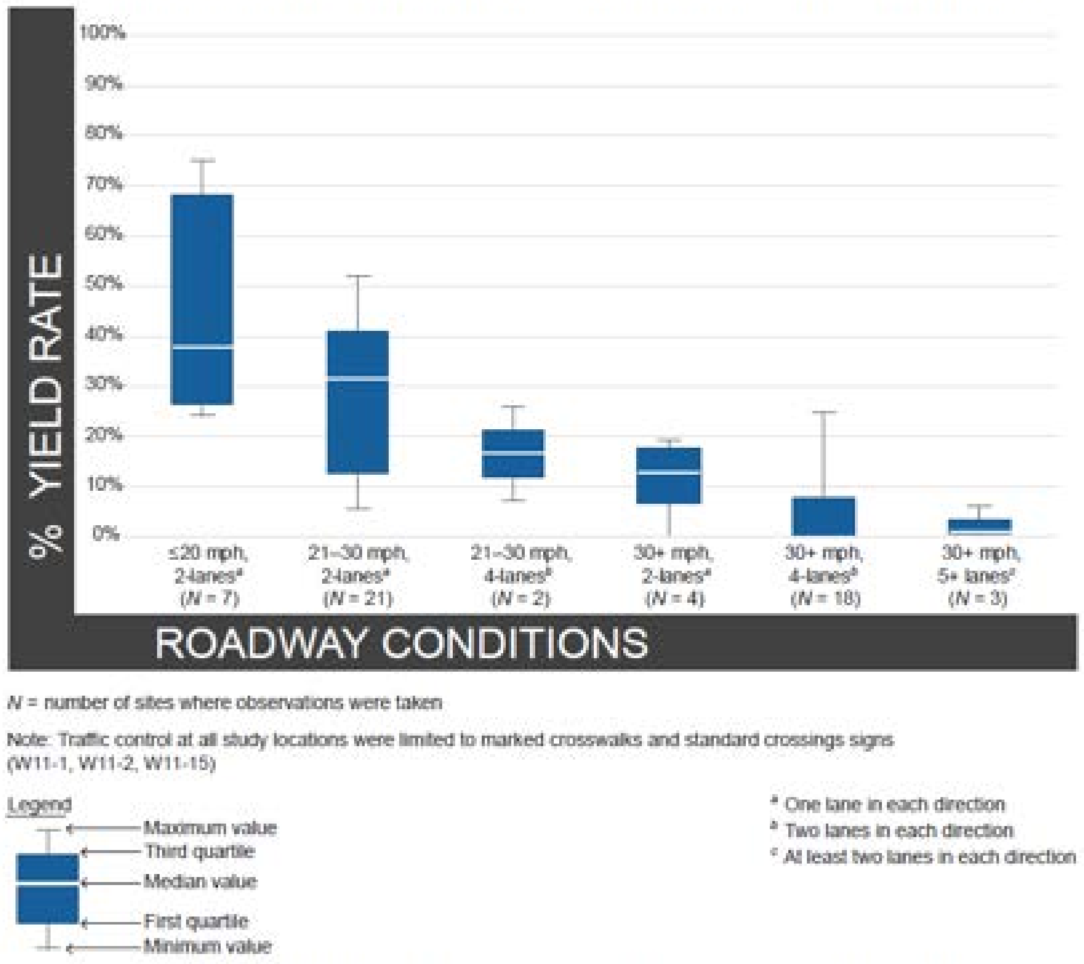

Figure 5-14: Motorist Yielding at Uncontrolled Crossings Based on Roadway Characteristics

TOOLE DESIGN

5.9.2.3 Apply Countermeasures to Improve Yielding

Tier 1: Signing & Markings

Tier 2: RRFB & Geometric Improvements

Tier 3: PHB, Signal, or Grade Separation

Table 5-15: Uncontrolled Crossing Evaluation

Uncontrolled Crossing Countermeasure Evaluation Table												
Roadway Type	Vehicle ADT < 9,000			Vehicle ADT 9,000 - 12,000			Vehicle ADT 12,000 - 15,000			Vehicle ADT > 15,000		
Number of Travel Lanes and Median Type	Speed Limit (mph)											
	≤30	35	40≥[a]	≤30	35	40≥[a]	≤30	35	40≥	≤30	35	40≥
2 Lanes[b]	1	1	2	1	1	2	1	1	3	1	2	3
3 Lanes with Raised Median[b]	1	1	2	1	1	2	1	2	3	2	2	3
3 Lanes without Raised Median[b,c]	1	1	2	1	2	2	2	3	3	2	3	3
4 Lanes with Raised Median[c,d]	1	1	2	1	2	2	2	3	3	3	3	3
4+ Lanes without Raised Median	1	2	3	2	2	2	3	3	3	3	3	3

Notes:

[a] Where the speed limit exceeds 40 mph, Tier 3 should be considered.

[b] 1 lane in each direction.

[c] Raised medians must be at least 6 ft wide to serve pedestrians. See Figure 2-4 for different bicycle lengths to serve bicyclists. Where median width is less than these values, review category of 4+ lanes without raised median.

[d] 2 lanes in each direction.

TOOLE DESIGN

Section 5.10 – Geometric Design Treatments to Improve Intersection Safety

5.10.1 Medians and Pedestrian Refuge Islands; Hardened Centerlines

5.10.2 Curb Extensions

5.10.3 Curb Radius

5.10.4 Mountable Truck Aprons

5.10.5 Raised Crossings

5.10.6 Multiple Threat Crossing Treatments

5.10.7 Bike Ramps

5.10.8 Directional Indicators

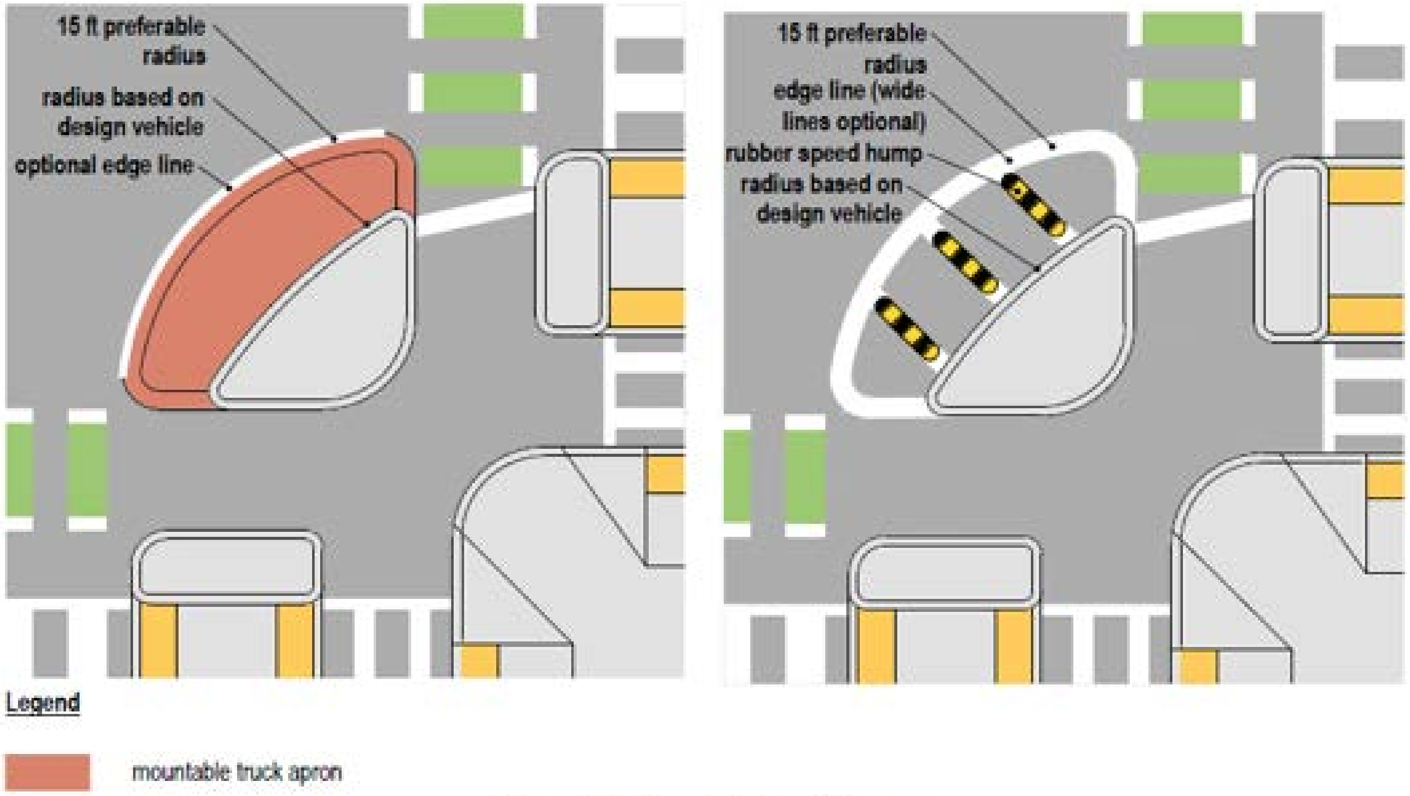

Figure 5-18: Mountable Truck Apron

TOOLE DESIGN

Section 5.10 – Geometric Design Treatments to Improve Intersection Safety

5.10.1 Medians and Pedestrian Refuge Islands; Hardened Centerlines

5.10.2 Curb Extensions

5.10.3 Curb Radius

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5.10.8 Directional Indicators

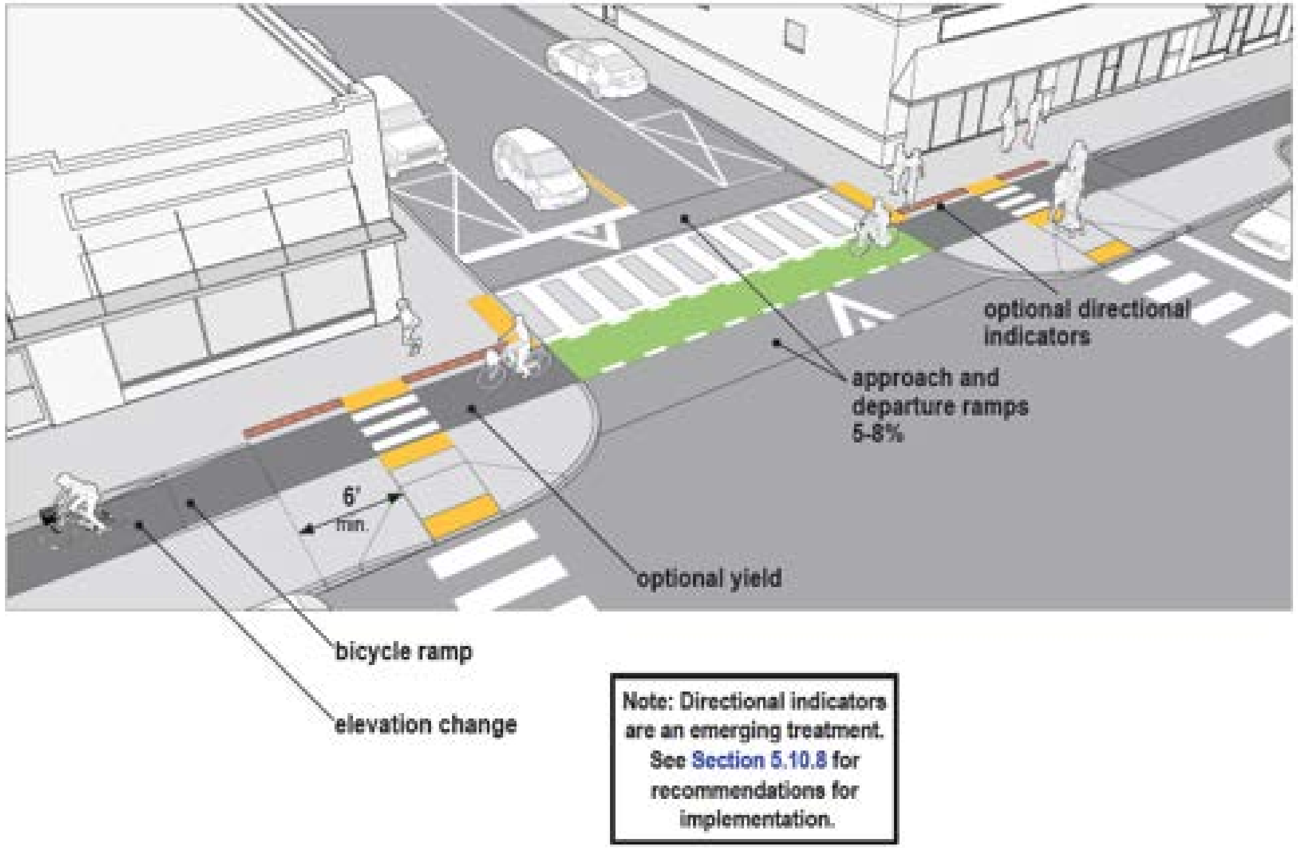


Figure 5-20: Raised Side Street Crossing

TOOLE DESIGN

Section 5.10 – Geometric Design Treatments to Improve Intersection Safety

5.10.1 Medians and Pedestrian Refuge Islands; Hardened Centerlines

5.10.2 Curb Extensions

5.10.3 Curb Radius

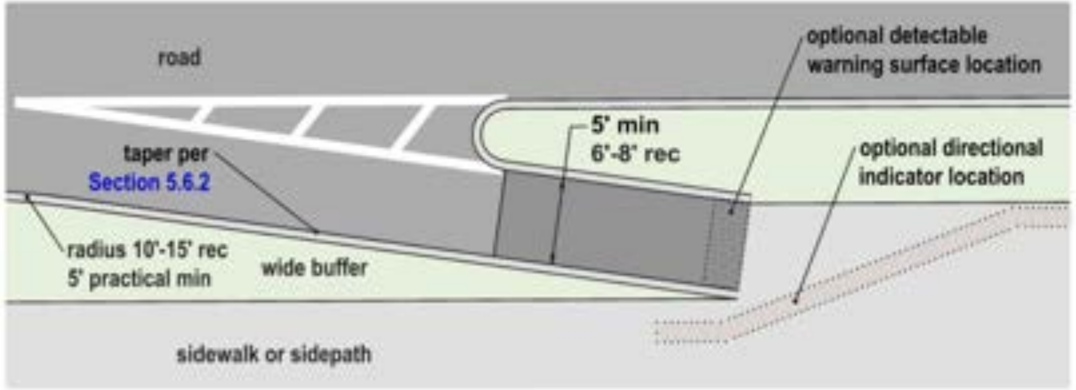
5.10.4 Mountable Truck Aprons

5.10.5 Raised Crossings

5.10.6 Multiple Threat Crossing Treatments

5.10.7 Bike Ramps

5.10.8 Directional Indicators



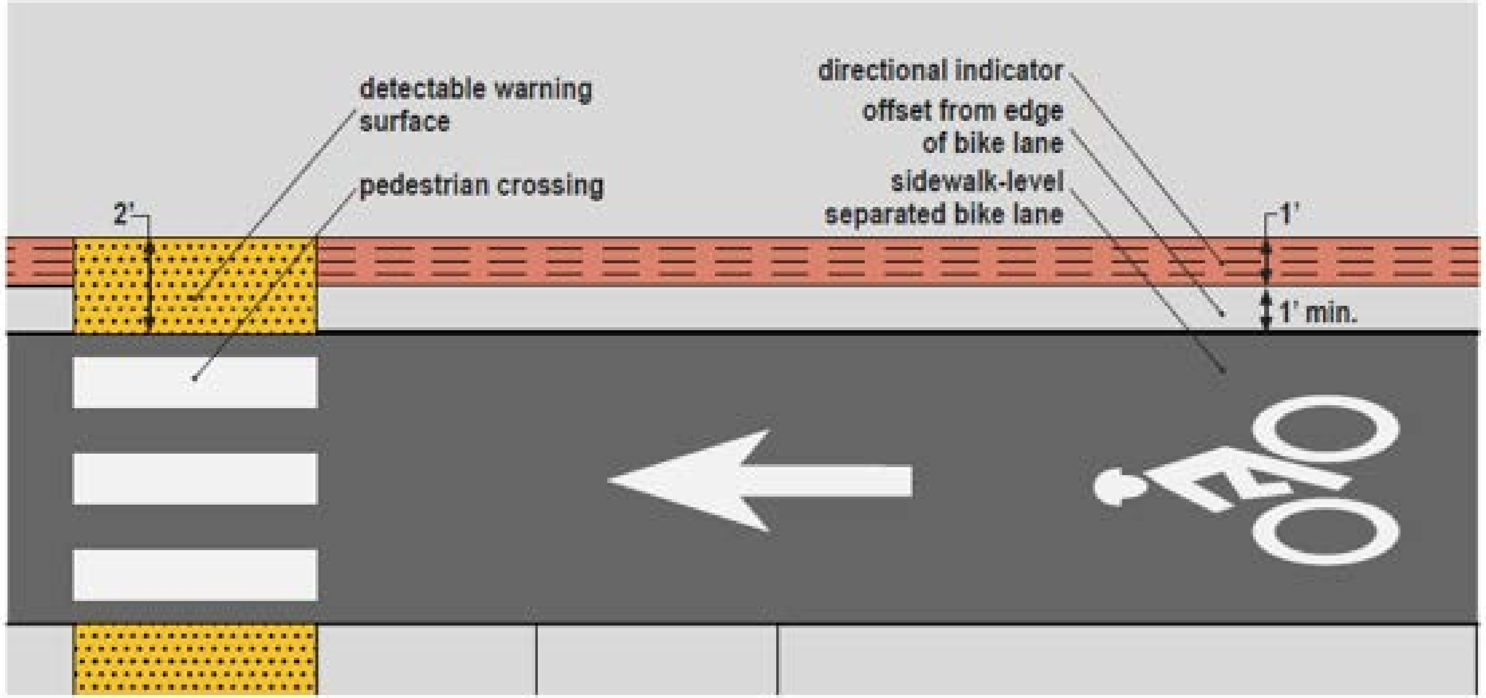
Detail 1—Preferred bicycle ramp alignment with wide sidewalk buffer

TOOLE DESIGN

5.10.8 Directional Indicators

Per ISO 23599 the width of the directional indicator (DI) can vary based on use:

- If perpendicular to the pedestrian path of travel (for example to direct a pedestrian towards a mid-block crossing or transit stop), it must be a minimum width of 2 ft to be detectable.
- If parallel to the pedestrian path of travel, it can be as narrow as 1 ft.
- At some locations (such as near intersections) pedestrian paths may interact with directional indicators both parallel and perpendicular, and in these situations the wider width should be used.



Note: Directional indicators are an emerging treatment. See Section 5.10.8 for recommendations for implementation.

Figure 5-24: Sidewalk-Level Separated Bike Lane with Directional Indicator

TOOLE DESIGN

5.11.5. Turning Vehicles Yield to Pedestrians/Bicyclists Signs

The use of the sign should be limited to the following:

- Crossings where turning motor vehicle volumes exceed 50 vehicles/hour.
- Locations where there is a documented problem with motorists failing to yield.
- Locations with inadequate sight lines and other mitigations are not feasible.
- New installations of left side bicycle lanes or two-way bikeways where counterflow bicycle travel may be unexpected.

A TURNING VEHICLES YIELD TO (or STOP FOR) BICYCLISTS (OR PEDESTRIANS) sign (R10-15 series) that uses a bicycle and pedestrian symbol is an experimental design. Experimental approval from FHWA is required to use this traffic control device (see Figure 5-29). See Section 1.6.1 for guidance on requests to experiment.

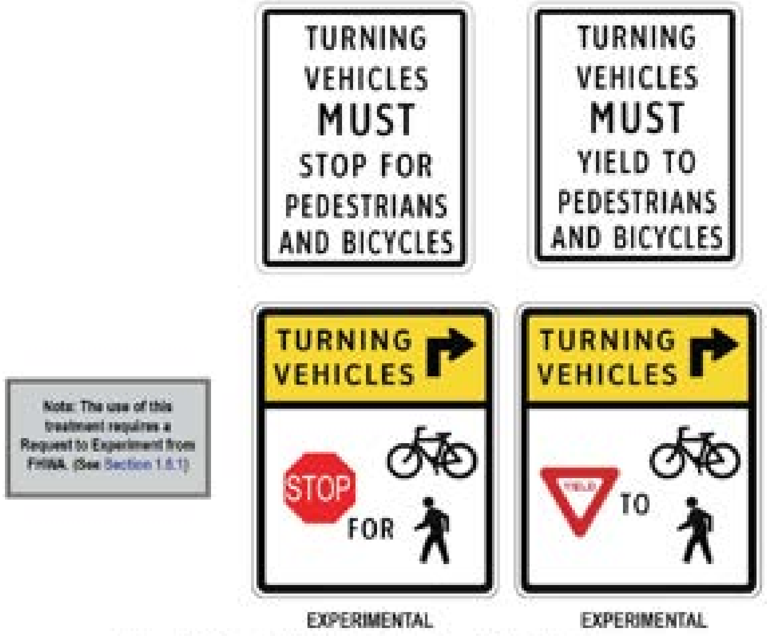


Figure 5-29: Turning Vehicles Yield to (or Stop for) Bicyclists Signs

TOOLE DESIGN

5.12 Pavement Markings

5.12.7.2 Bicycle Crossings with Parallel Pedestrian Crossings

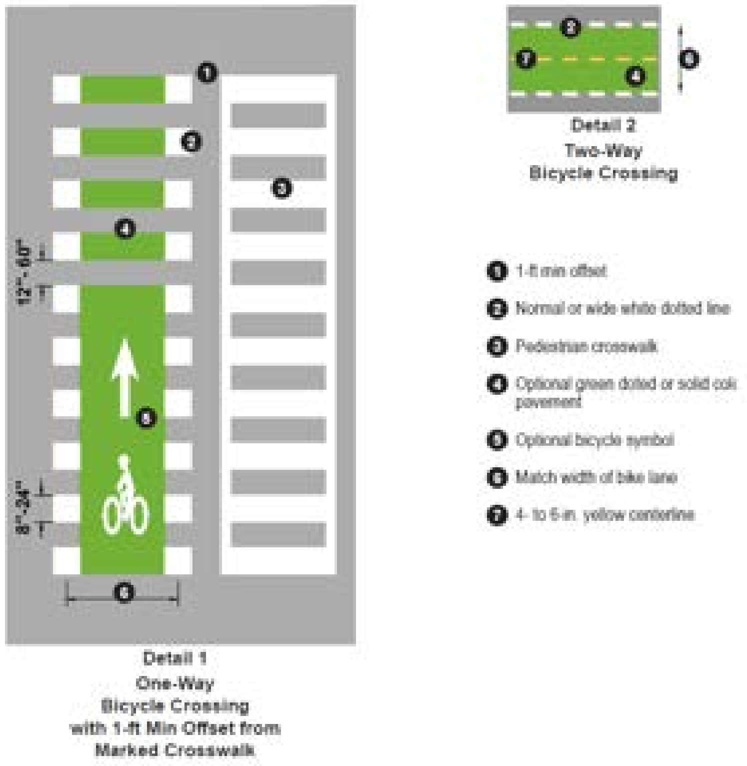
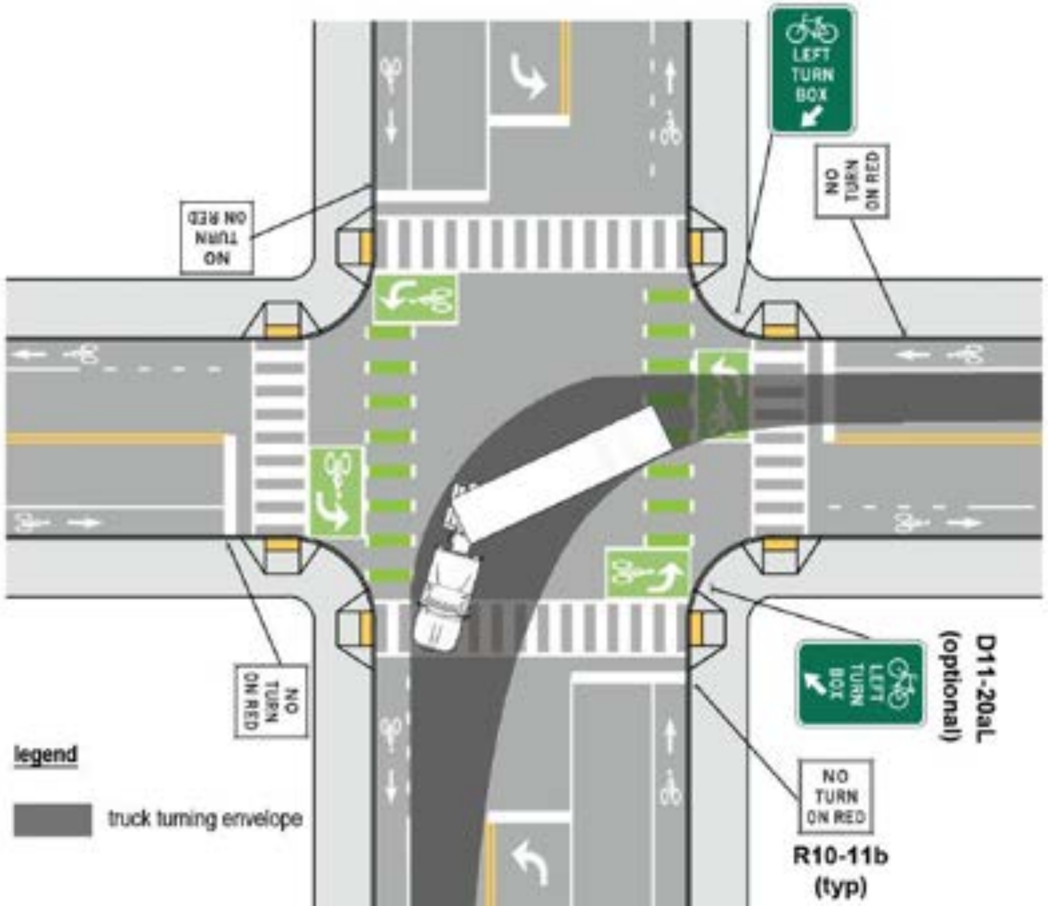


Figure 5-35: Bicycle Crossing Pavement Markings

5.12.9. Two-Stage Bicycle Turn Box



TOOLE DESIGN

Chapter 6 – Shared Use Paths

6.1 Introduction

6.2 Shared Use Path Users

6.3 Side Path Considerations

6.4 Path Width Considerations

6.5 Design Speed

6.6 General Design Considerations

6.7 Shared Use Path Intersections and Transitions

6.8 Design Considerations to Promote Personal Security

6.9 Shared Use Path Entrance and Wayside Amenities

Chapter 6
SUP Width (Two-way)

6.4.3. Recommended Shared Use Path Widths

Table 6-3: Recommended Shared Use Path Widths* to Achieve SUP LOS "C"

Shared Use Path Operating Widths and Operational Lanes*					
SUPLOS "C" Peak Hour Volumes	Recommended Operational Lanes	Practical Minimum	Recommended Lower Limit	Recommended Upper Limit	Practical Maximum
150 to 300	2	8 ft	10 ft	12 ft	13 ft
300 to 500	3	11 ft	12 ft	15 ft	16 ft
500 to >600	4	15 ft	16 ft	20 ft	None

*Typical Mode Split is 55% adult bicyclists, 20% pedestrians, 10% runners, 10% in-line skaters, and 5% child bicyclists

11' wide provides three (3) operational lanes

TOOLE DESIGN

6.4.2. Shared Use Path Level of Service

Table 6-1: Shared Use Path Operating Conditions Based on Level of Service Criteria

Shared Use Path Level of Service (SUPLOS) and Operating Conditions	
SUPLOS	**Peak Operating Conditions**
A. Excellent	A significant ability to absorb more users across all modes is available.
B. Good	A moderate ability to absorb more users across all modes is available.
C. Fair	Path is close to functional capacity with minimal ability to absorb more users.
D. Poor	Path is at its functional capacity. Additional users will create operational and safety problems.
E. Very Poor	Path operating beyond its functional capacity resulting in conflicts and people avoiding the path.
F. Failing	Path operating beyond functional capacity resulting in significant conflicts and people avoiding the path.

Table 6-2: Shared Use Path Level of Service Look-Up Table, Typical Mode Split

Shared Use Path Level of Service Look-Up Table, Typical Mode Split*										
Shared Use Path Peak Hour Volume	Shared Use Path Width (ft)									
	8	10	11	12	14	15	16	18	20	≤ 25
50	B	B	B	B	B	A	A	A	A	A
100	D	C	B	B	B	A	A	A	A	A
150	D	C	B	B	B	A	B	A	A	A
200	D	D	C	B	B	A	B	A	A	A
300	E	D	C	C	C	B	B	B	B	A
400	F	E	D	D	C	C	C	B	B	A
500	F	F	D	D	D	C	C	C	C	A
600	F	F	E	E	E	D	D	C	C	A
800	F	F	F	F	F	E	E	E	E	A
1,000	F	F	F	F	F	E	F	F	F	A
≥ 1,200	F	F	F	F	F	F	F	F	F	A

*Assumptions:

1. Mode split is 55 percent adult bicyclists, 20 percent pedestrians, 10 percent runners, 10 percent in-line skaters, and 5 percent child bicyclists.
2. An equal number of trail users travel in each direction (the model uses a 50 percent–50 percent directional split).
3. Trail volume represents the actual number of users counted in the field (the model adjusts this volume based on a peak hour factor of 0.85).
4. Trail has a centerline.

TOOLE DESIGN

6.4.4. Separation of Pedestrians and Bicyclists

6.4.4.1 Land Use Considerations Where Separation is Desirable

6.4.4.2 Volume Thresholds Where Separation is Desirable

Should be considered when:

- Level of Service is projected to be at or below level "C."
- Pedestrians can reasonably be anticipated to be 30% or more of the volume

6.4.4.3 Separation Strategies

6.4.4.4 Accessibility Considerations

Figure 6-3: Burke-Gilman Shared Use Path (2008) and Separated Paths (2021), Seattle, WA

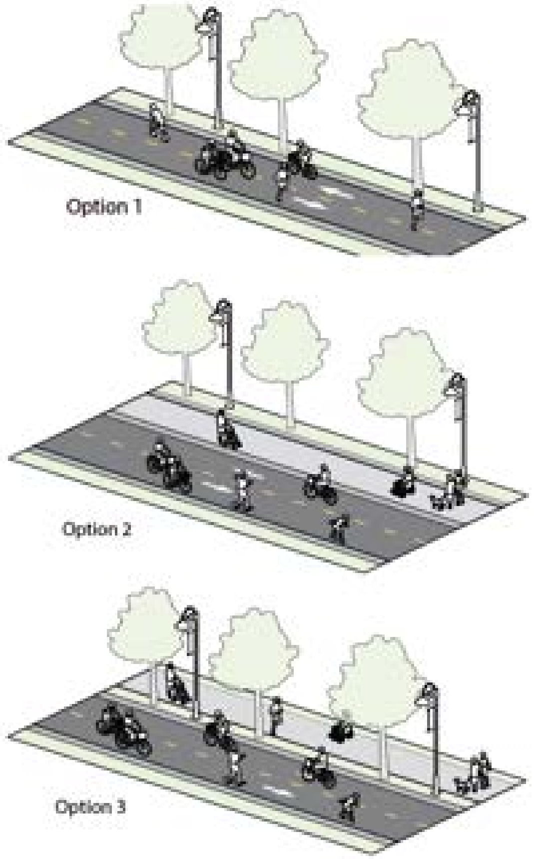


Figure 6-4: Options for Separating Bicyclists and Other Wheeled Users from Pedestrians

TOOLE DESIGN

6.6. General Design Considerations

6.6.1. Shy Distance, Clearances, and Shoulders

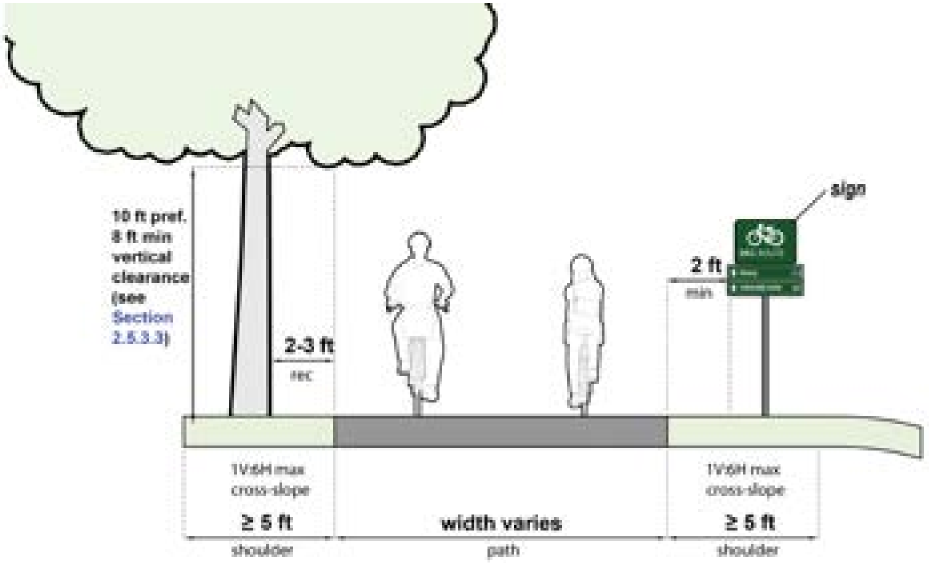


Figure 6-5: Shoulders and Shy Distance on Shared Use Paths

6.6.3. Horizontal Alignment

Table 6-5: Minimum Radii for Horizontal Curves at 20-Degree Lean Angles

Design Speed (mph)	Minimum Radii (ft) for Horizontal Curves at 20-Degree Lean Angles
8	12
10	18
12	27
14	36
16	47
18	60
20	74
25	115
30	166

6.6.4. Vertical Alignment

Table 6-7: Length of Crest Vertical Curve to Provide Sight Distance Equations

Length of Crest Vertical Curve to Provide Sight Equations

when $S > L$ $\quad L = 2S - \frac{200(\sqrt{h_1} + \sqrt{h_2})^2}{A}$

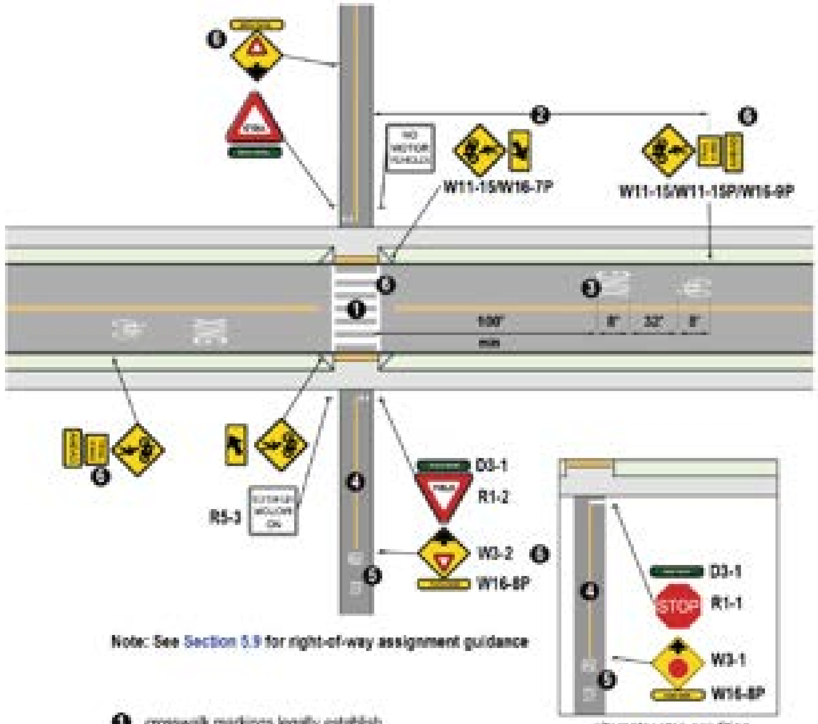
when $S < L$ $\quad L = \frac{AS^2}{100(\sqrt{2h_1} + \sqrt{2h_2})^2}$

where:

L	=	minimum length of vertical curve (ft)
A	=	algebraic grade difference (percent)
S	=	stopping sight distance for flat grade (ft)*
h_1	=	eye height (3.83 ft for a typical recumbent bicyclist)
h_2	=	object height (0 ft)

*See Tables 5-2 and 5-3.

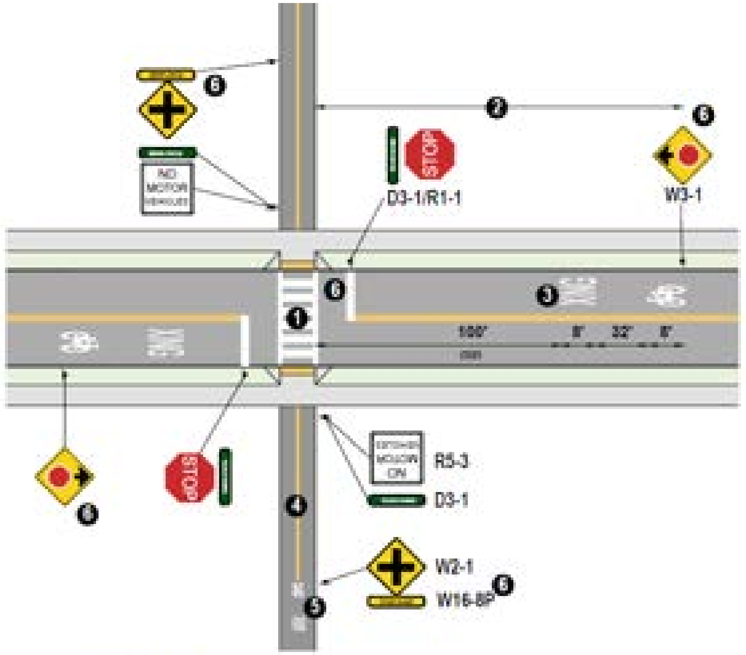
6.7. Shared Use Path Intersections and Transitions



Note: See Section 5.9 for right-of-way assignment guidance

1. crosswalk markings legally establish midblock pedestrian crossing
2. length varies: see MUTCD Table 2C-4
3. optional roadway markings
4. shared-use path centerline as needed
5. optional pathway markings and advance warning signage
6. optional advance warning signs; these signs are recommended where visibility to crossing is limited

Figure 6-13: Shared Use Path Stops or Yields



Note: See Section 5.9 for right-of-way assignment guidance

1. crosswalk markings legally establish midblock pedestrian crossing
2. length varies: see MUTCD Table 2C-3
3. optional roadway markings
4. shared-use path centerline as needed
5. optional pathway markings and signage
6. optional advance warning signs; these signs are recommended where visibility to crossing is limited

Figure 6-14: Road Stops

TOOLE DESIGN

Chapter 7 – Separated Bike Lanes and Side Paths

7.1 Introduction

7.2 General Design Considerations

7.3 Bike Lane Zone

7.4 Street Buffer Zone

7.5 Sidewalk Buffer Zone

7.6 Consideration for Zone Widths in Constrained Locations

7.7 Utility Considerations

7.8 Landscaping Considerations

7.9 Separated Bikeway and Side Path Intersection Design

7.10 Transitions Between Facilities

7.11 Raised Bike Lanes

7.2. General Design Considerations

The cross section of a separated bike lane comprises three distinct zones (see Figure 7-1):

1. **Bike lane**—The bike lane is the space in which the bicyclist operates. It is located between the street buffer and the sidewalk buffer.
2. **Street buffer**—The street buffer separates the bike lane or side path from motor vehicle traffic.
3. **Sidewalk buffer**—The sidewalk buffer separates the bike lane from the sidewalk.

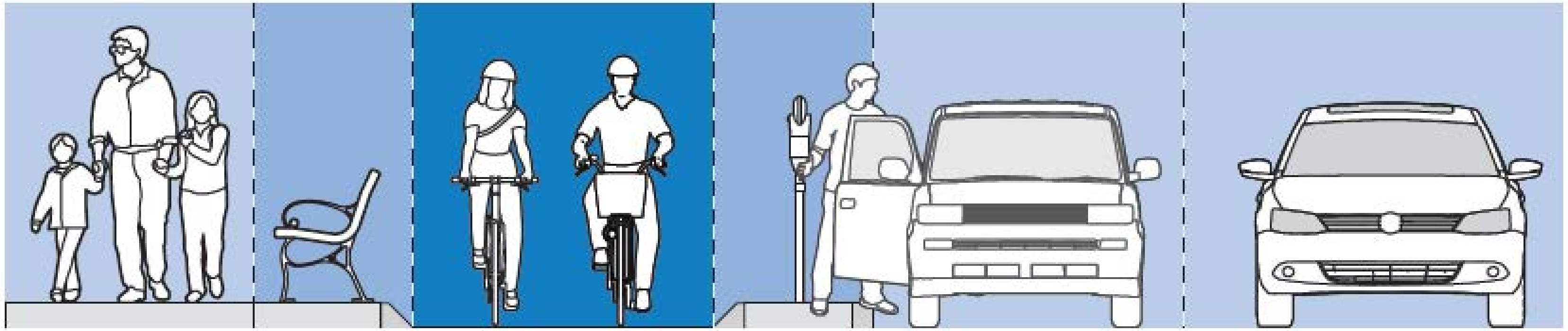


Figure 7-1: Separated Bike Lane Zones

7.2.2.3 Intermediate-Level Separated Bike Lanes

curb reveal of 2-3 in. below sidewalk elevation is recommended to”

- provide vertical separation to the adjacent sidewalk, and
- provide a detectable edge for pedestrians with vision disabilities

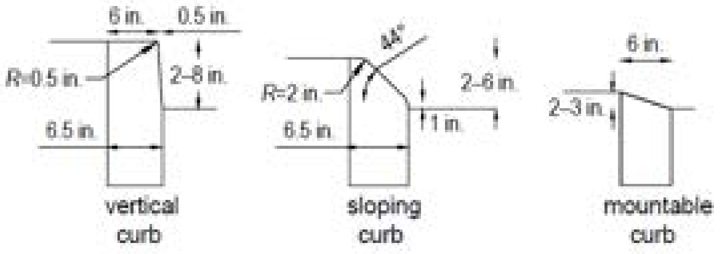


Figure 7-5: Curb Types for Separated Bike Lanes

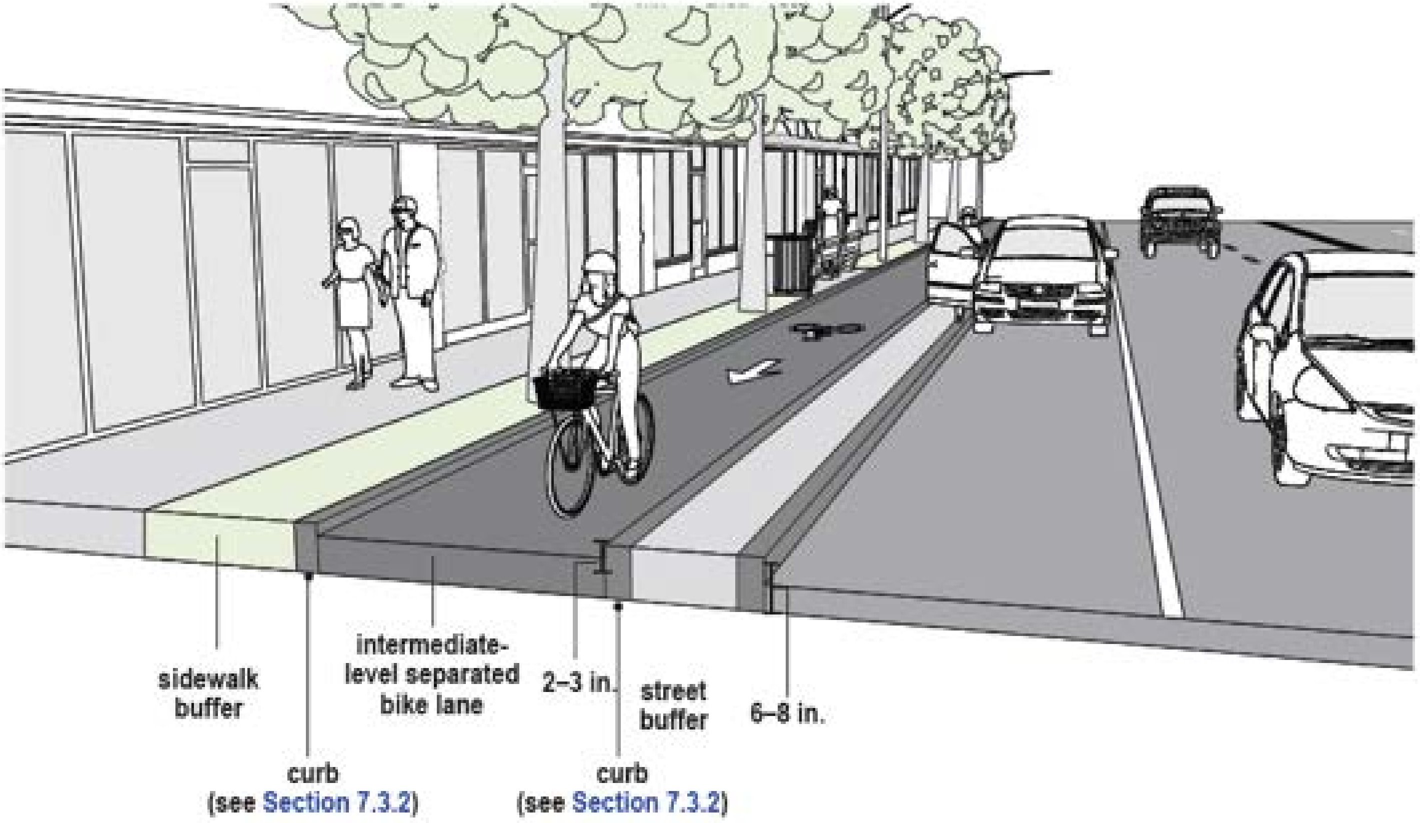


Figure 7-4: Intermediate-Level Separated Bike Lane

TOOLE DESIGN

Section 7.3.4 – SBL Width (One-way)

Table 7-3: One-Way Separated Bike Lane Widths Based on Existing or Anticipated Volumes

Peak Hour Directional Bicyclist Volume	One-Way Separated Bike Lane Width (ft) Recommended Values		
	Between Vertical Curbs without Gutter	Adjacent to One Vertical Curb	Between Sloped Curb, at Sidewalk Level, or Adjacent to Curb with Gutter
<150	6.5–8.5	6–8	5.5–7.5
150–750	8.5–10	8–9.5	7.5–9
>750	≥10	≥9.5	≥9
Practical Minimum*	4.5	4	4

*Peak Hour Directional Bicyclist Volume not applicable

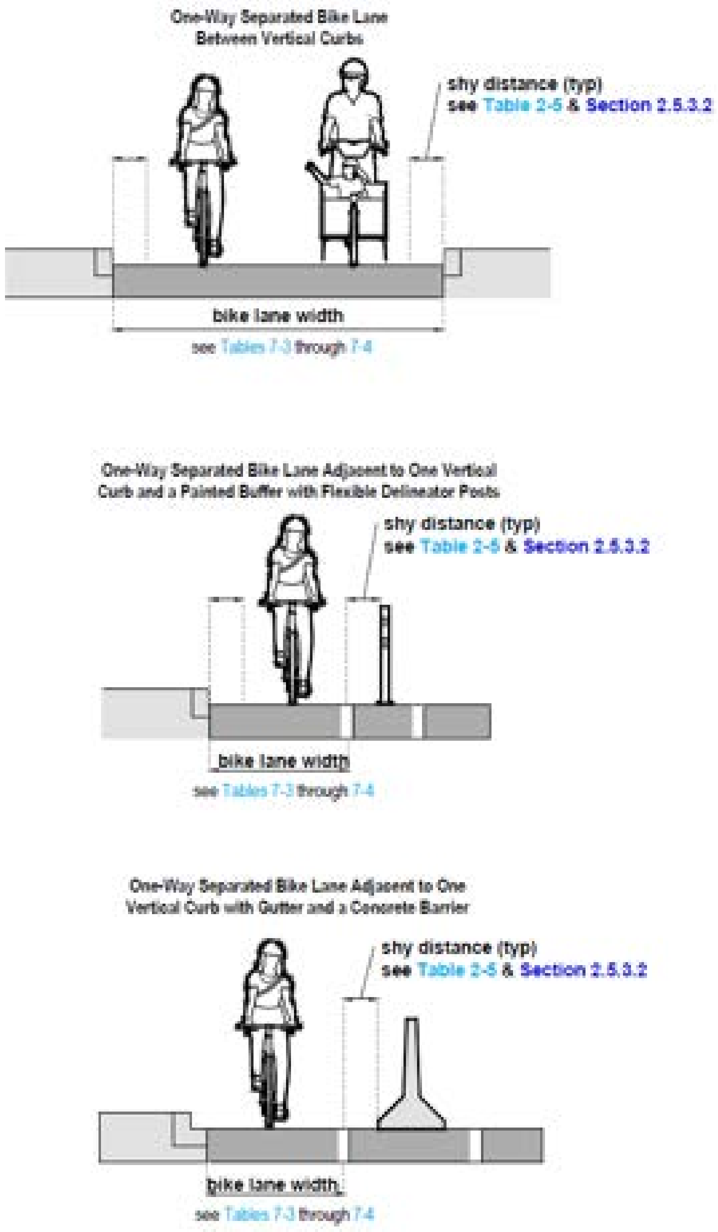


Figure 7-7: Separated Bike Lane Width

TOOLE DESIGN

7.7.1. Drainage and Stormwater Management

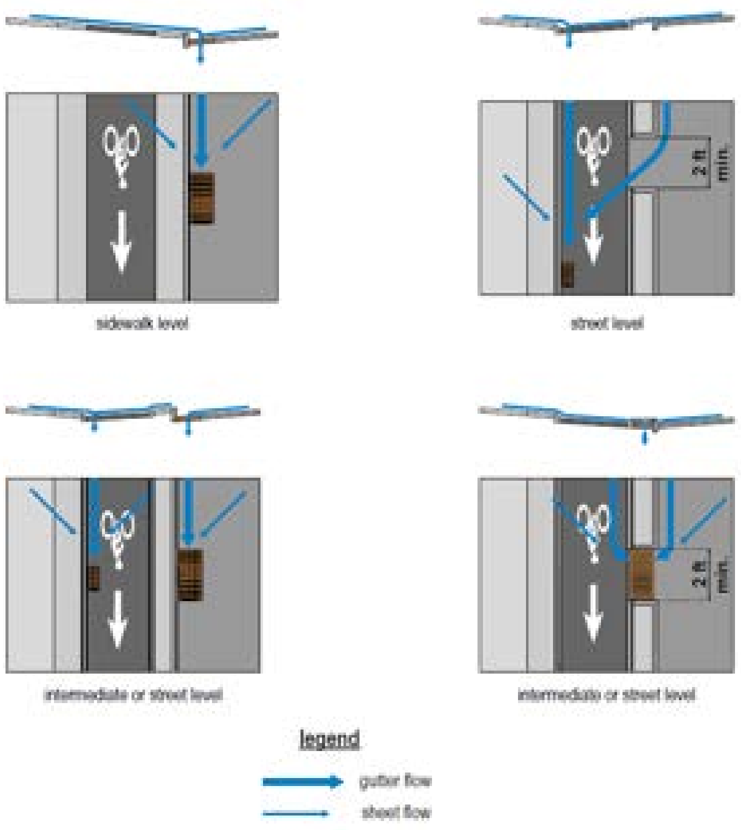


Figure 7-11: Examples of Separated Bike Lane Drainage Options

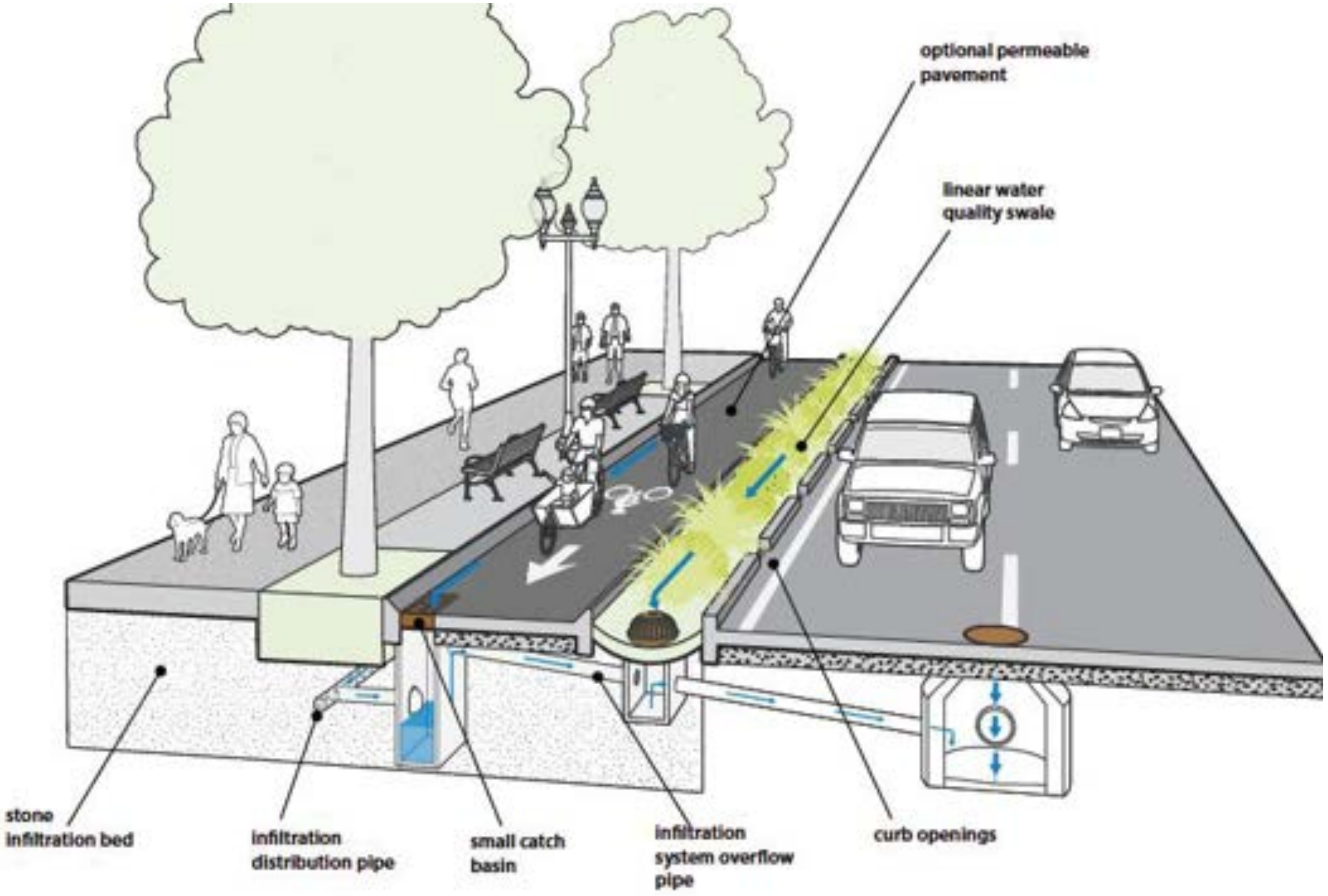
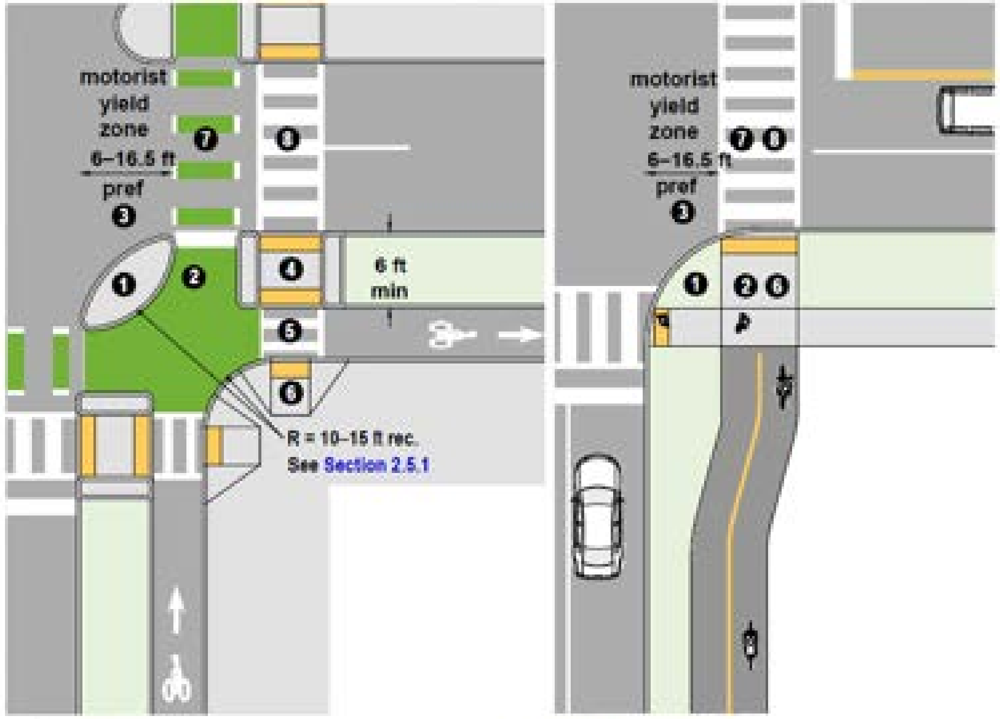


Figure 7-10: Green Stormwater Infrastructure in an Urban Street Context

TOOLE DESIGN

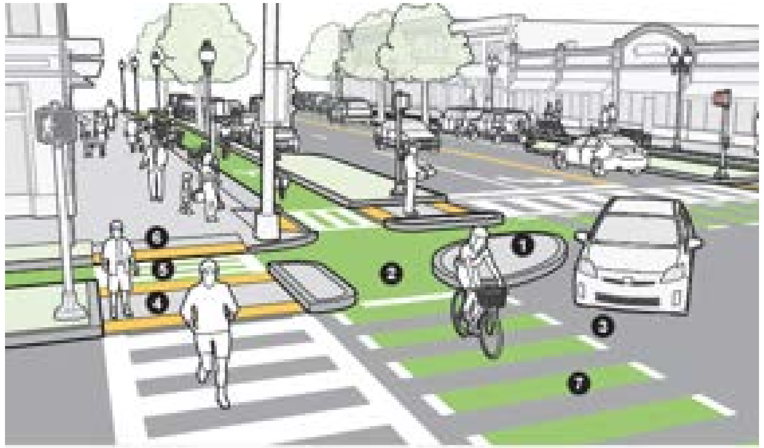
7.9. Separated Bike Lane and Side Path Intersection Design

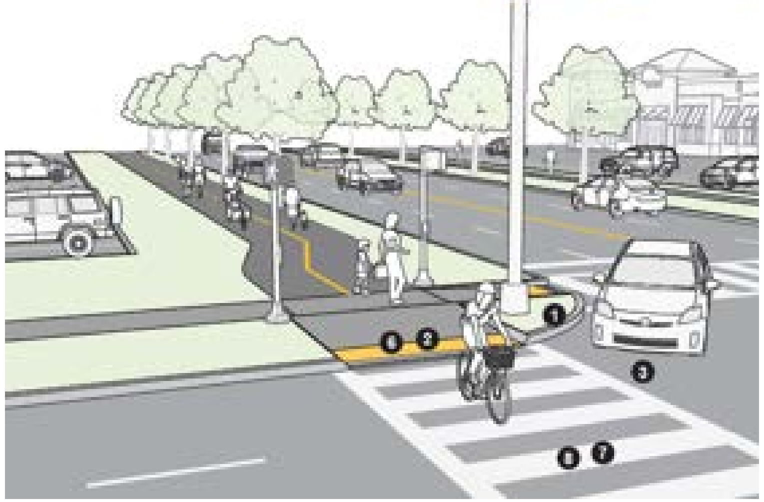
- 7.9.1. Minimizing Exposure to Conflicts
- 7.9.2. Reducing Speeds at Conflict Points
- 7.9.3. Transitions between Elevations
- 7.9.4. Right-of-Way Priority
- 7.9.5. Sight Distance
- 7.9.6. Restricting Motor Vehicles



1. corner island
2. forward bicycle queuing area
3. motorist yield zone
4. pedestrian refuge island
5. pedestrian crossing of the separated bike lane
6. pedestrian curb ramp
7. bicycle crossing of travel lanes
8. pedestrian crossing of travel lanes

Figure 7-13: Protected Intersection Design for Separated Bike Lanes and Side Paths





TOOLE DESIGN

7.9.7.1 Corner Island

Benefits:

- forward bicycle queuing area
- space for turning vehicles to wait
- reduces crossing distances
- reduces motorist turning speeds
- can reduce bicyclist speeds by adding deflection to the bike lane or side path

Figure 7-15: Corner Island with Flexible Delineator Posts (Source: Carl Sundstrom, PE, Office of Bicycle and Pedestrian Programs, New York City Department of Transportation)

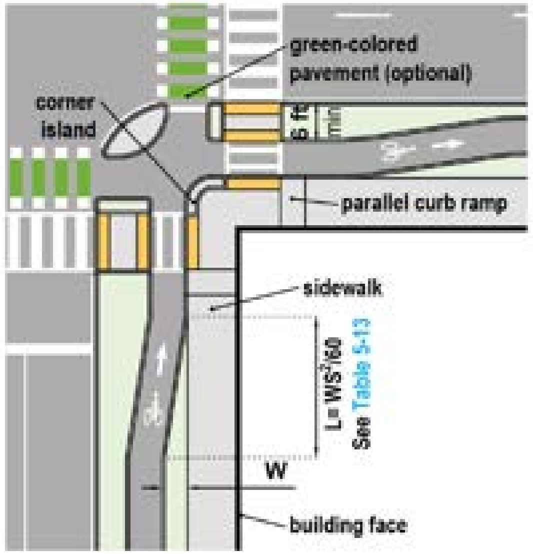


Figure 7-18: Bend-Out Example

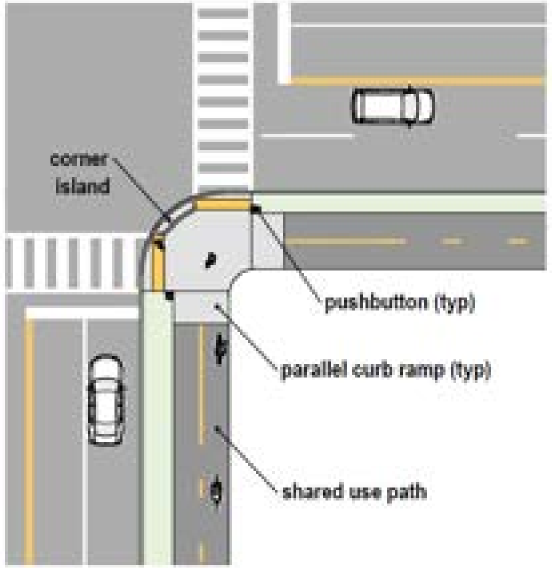


Figure 7-17: Side Path Curb Ramps at Constrained Intersection

TOOLE DESIGN

7.9.9. Intersection Design with Mixing Zones

Reduce speeds of motor vehicles entering the merge point to 20 mph or less:

- Minimize the length of the merge area
- Locate the merge point as close as practical to the intersection.
- Minimize the length of the storage portion of the turn lane.
- Provide a buffer and physical separation (e.g., flexible delineator posts) from the adjacent through lane after the merge area, if feasible.
- Highlight the conflict area with a green-colored pavement and dotted bike lane markings (see Figure 7-20), as necessary, or shared lane markings (see Figure 7-21).
- Raise the elevation of the turn lane at the start of the mixing zone.

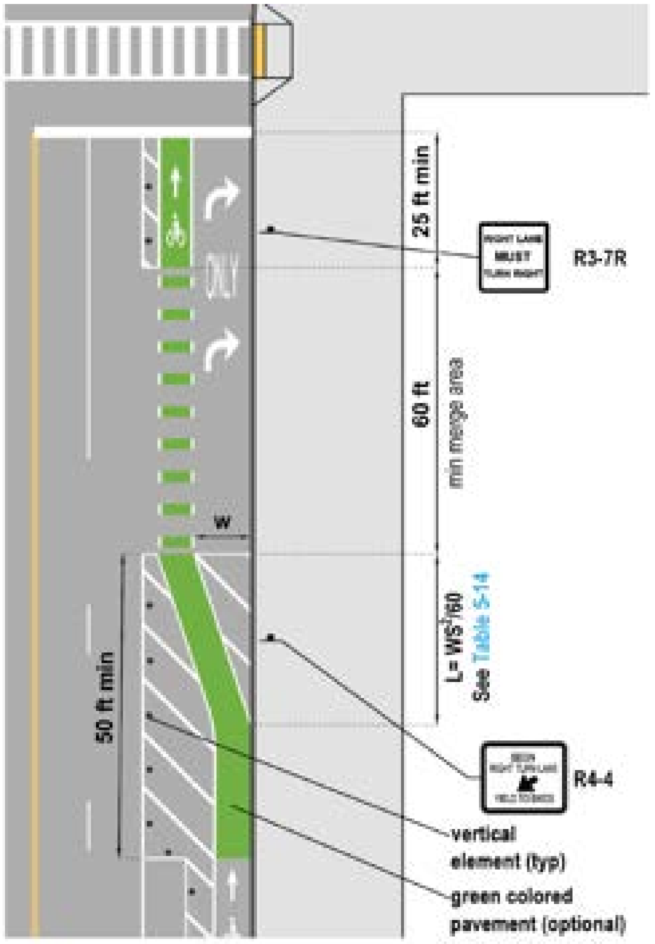


Figure 7-20: Angled Crossing Mixing Zone with Bike Lane

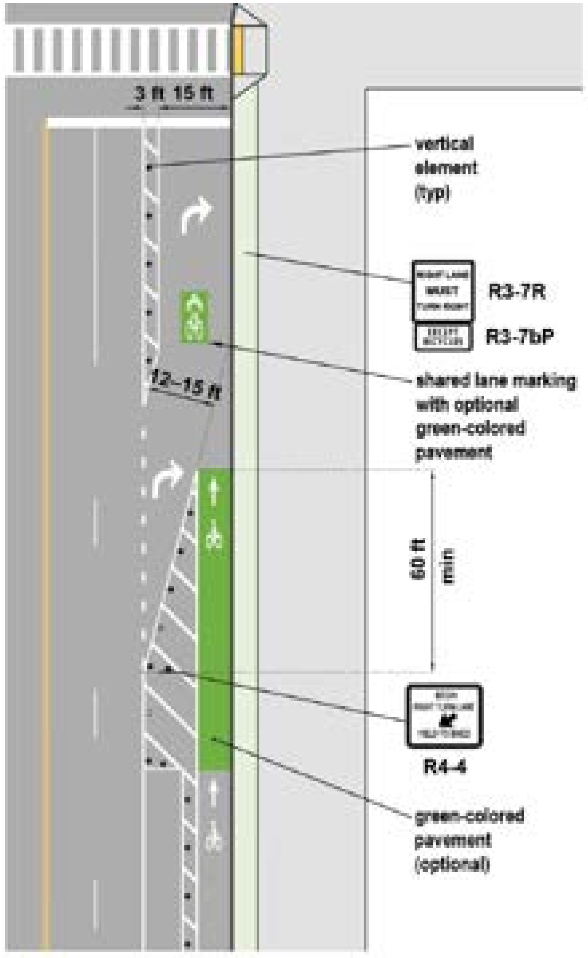
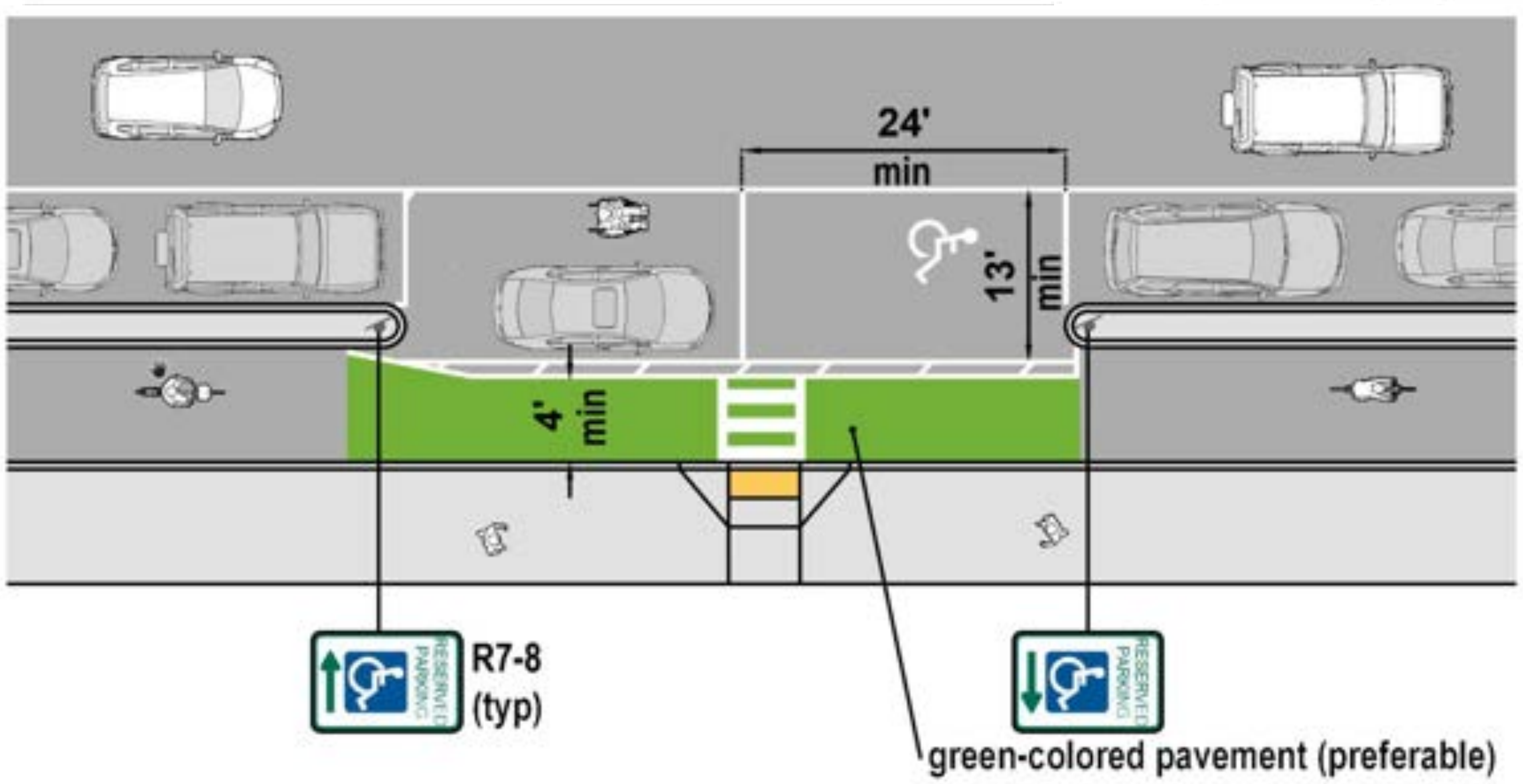
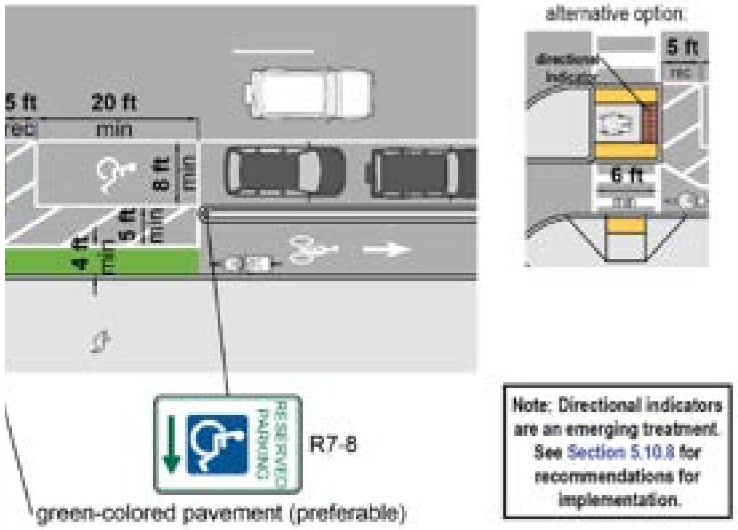


Figure 7-21: Angled Crossing Mixing Zone with Shared Lane

TOOLE DESIGN

7.9.12.1 Accessible Motor Vehicle Parking





:essible On-Street Motor Vehicle Parking at Intersections

TOOLE DESIGN

7.9.14. Transit Stops

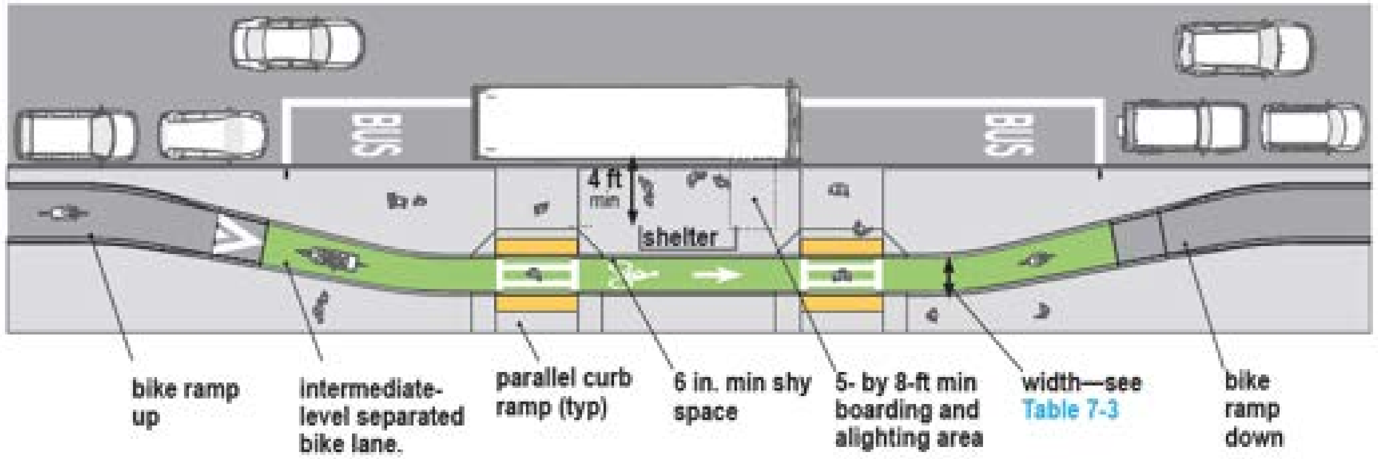
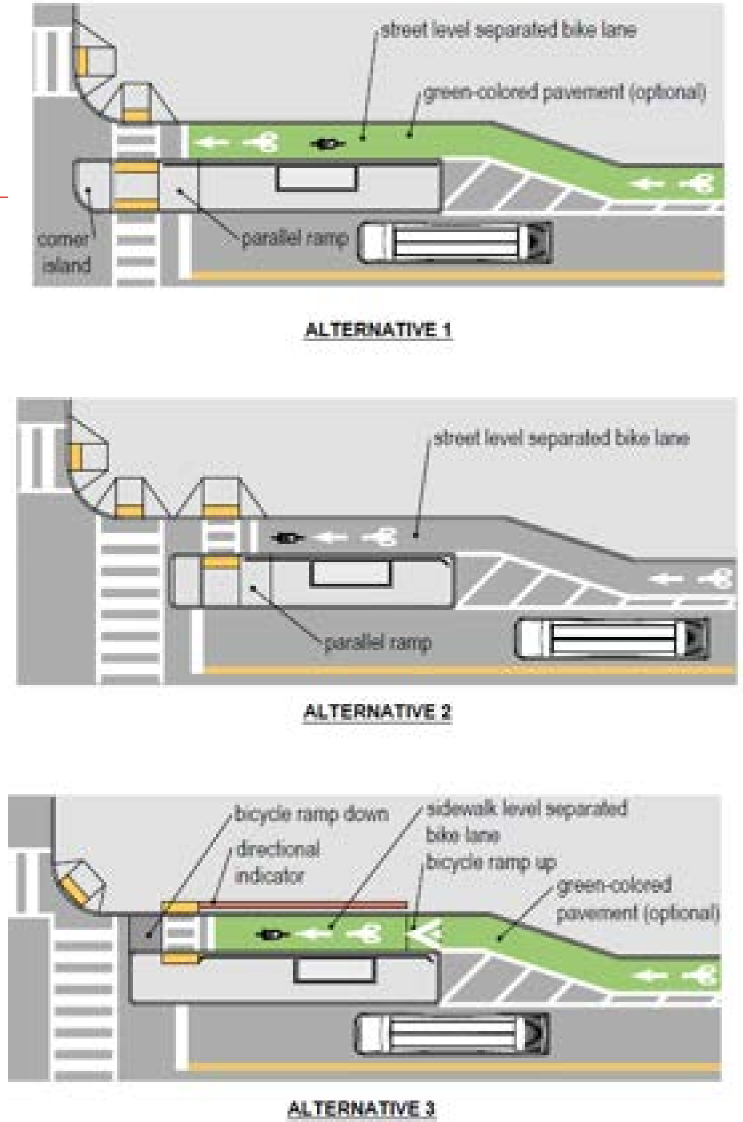


Figure 7-26: Example Configuration: Floating Transit Stop (Mid-Block)



Note: Directional indicators are an emerging treatment. See Section 5.10.8 for recommendations for implementation.

Figure 7-31: Example Configurations: Floating Transit Stop (Near-Side)

7.10. Transitions between Facilities

In general, it is preferable for a transition from a separated bike lane to a standard bicycle lane or shared lane to occur on the far side of the intersection.

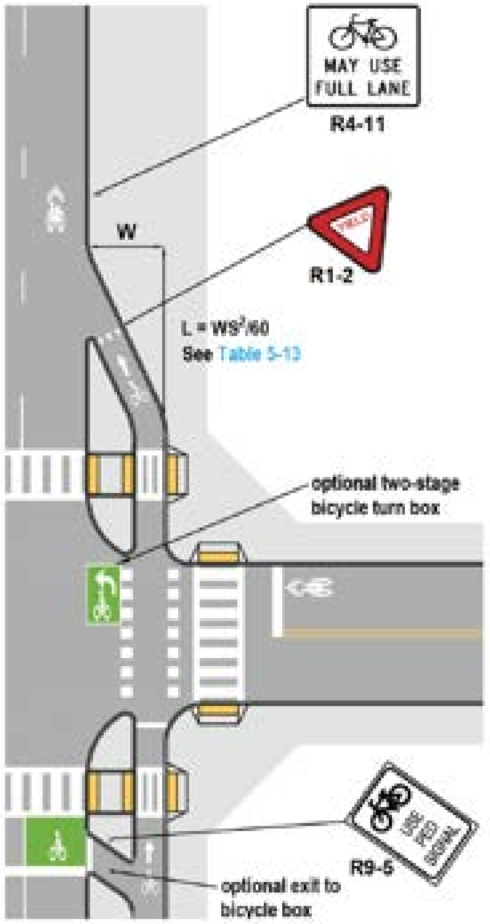


Figure 7-32: Transition to Shared Lane

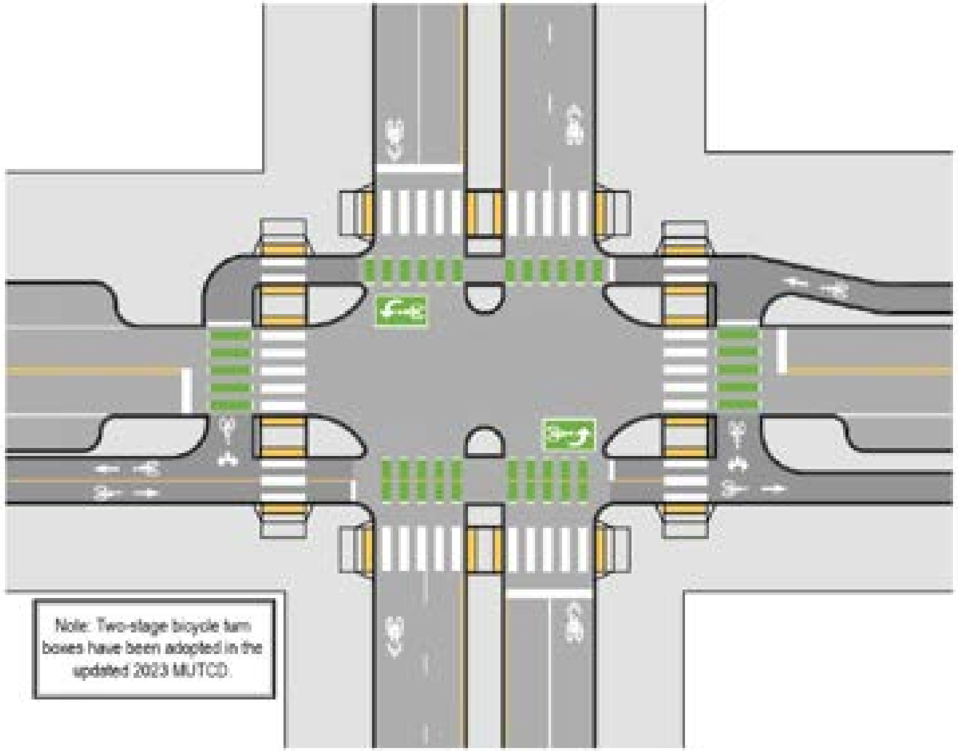


Figure 7-35: Transition from One-Way to Two-Way Separated Bike Lanes at Protected Intersection

TOOLE DESIGN

7.11. Raised Bike Lanes

Table 7-5: Raised Bike Lane Widths

Raised Bike Lane Widths				
Bike Lane Context	Practical Minimum (ft)	Recommended Lower Limit (ft)	Recommended Upper Limit (ft)[2]	Practical Maximum (ft)[2]
Intermediate level or sidewalk level raised bike lane[1]	5	6.5	8	10

[1]Raised bike lane widths are exclusive of the gutter unless the gutter is integrated into the full widths of the bike lane.

[2]Separated bike lane with a street buffer may be preferable to a curb-attached, wide raised bike lane.

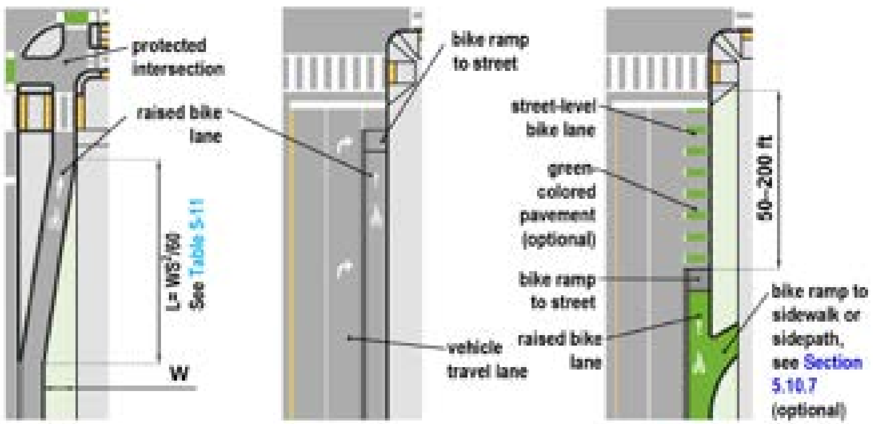


Figure 7-40: Raised Bike Lane Transitions at Intersections

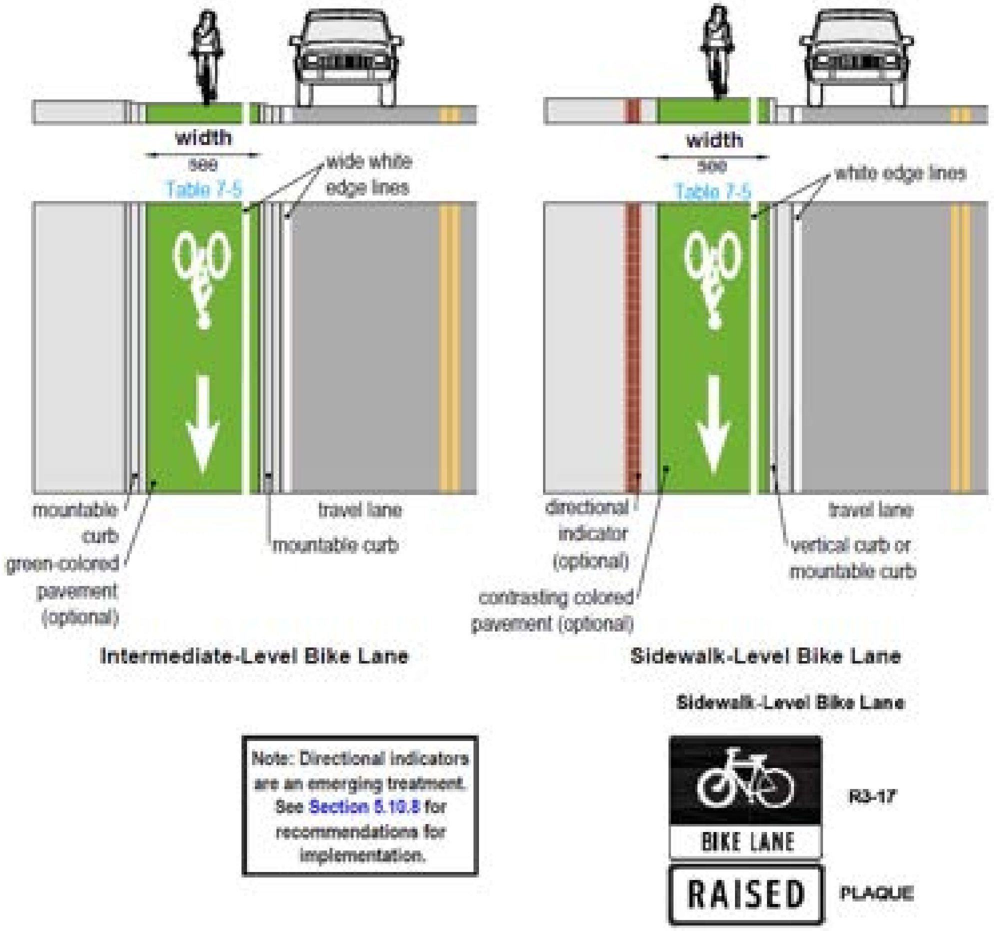


Figure 7-39: Intermediate-Level and Sidewalk-Level Raised Bike Lanes

Chapter 8 – Bicycle Boulevard Planning and Design

8.1 Introduction

8.2 Bicycle Boulevard Principles

8.3 Bicycle Boulevard Minimum Design Elements

8.4 Traffic Calming Strategies (Speed Management)

8.5 Traffic Diversion Strategies (Volume Management)

8.6 Traffic Control for Minor Street Crossings

8.7 Traffic Control for Major Street Crossings

Section 8.2 – Bicycle Boulevard Principles

Bicycle Boulevards are not just signed bike routes.

Principles that set them apart from local streets include:

- 8.2.1. Manage motorized through traffic volumes and speeds
- 8.2.2. Prioritize right-of-way at local street crossings
- 8.2.3. Provide safe and convenient crossings at major streets

Minimize Motorized Through Traffic Volumes and Speed Differential			
	Hourly Traffic Volume	Daily Traffic Volume	Speed
Preferred	50 vehicles/hr	1,000 ADT	15 mph
Acceptable	75 vehicles/hr	2,000 ADT	20 mph
Maximum	100 vehicles/hr	3,000 ADT	25 mph

Major Street Crossings (opportunities per hour)	
Preferred	120
Minimum	60

TOOLE DESIGN

8.4. Traffic Calming Strategies (speed management)

Figure 8-5: Example of a Chicane Treatment on a Two-Way Street Created by a Median and Curb Extensions

Figure 8-6: Example of a Chicane Treatment Created by Alternating Parking from One Side of the Street to the Other

TOOLE DESIGN

8.5. Traffic Calming Strategies (volume management)

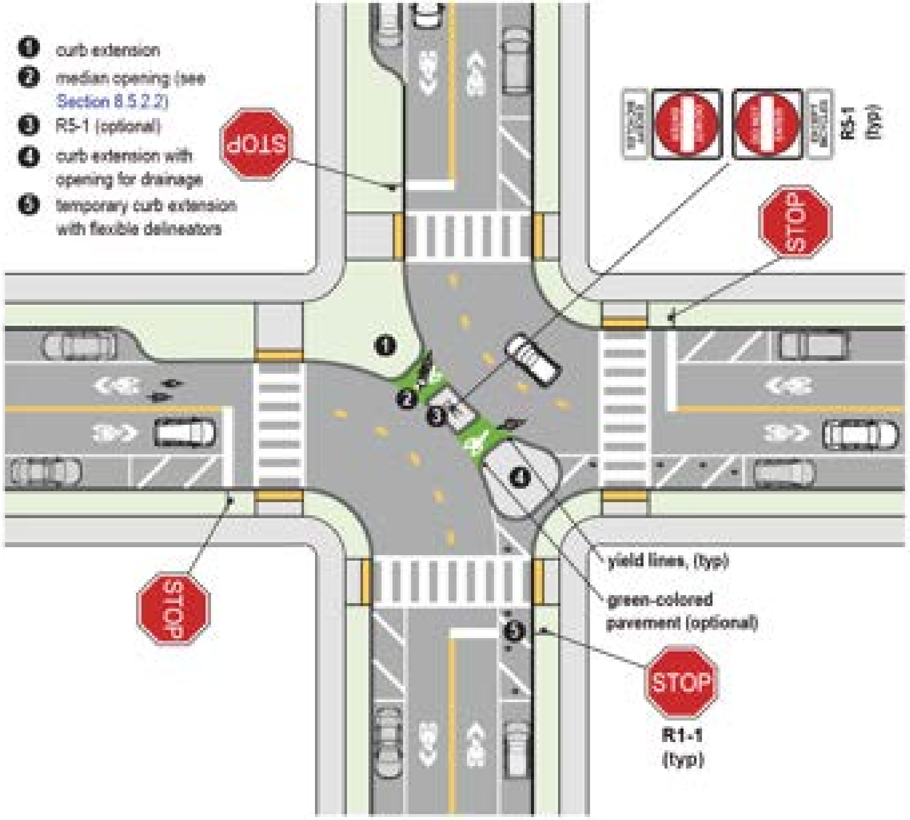


Figure 8-12: Example of a Median Used to Create a Diagonal Diverter at Intersection of Two Local Streets

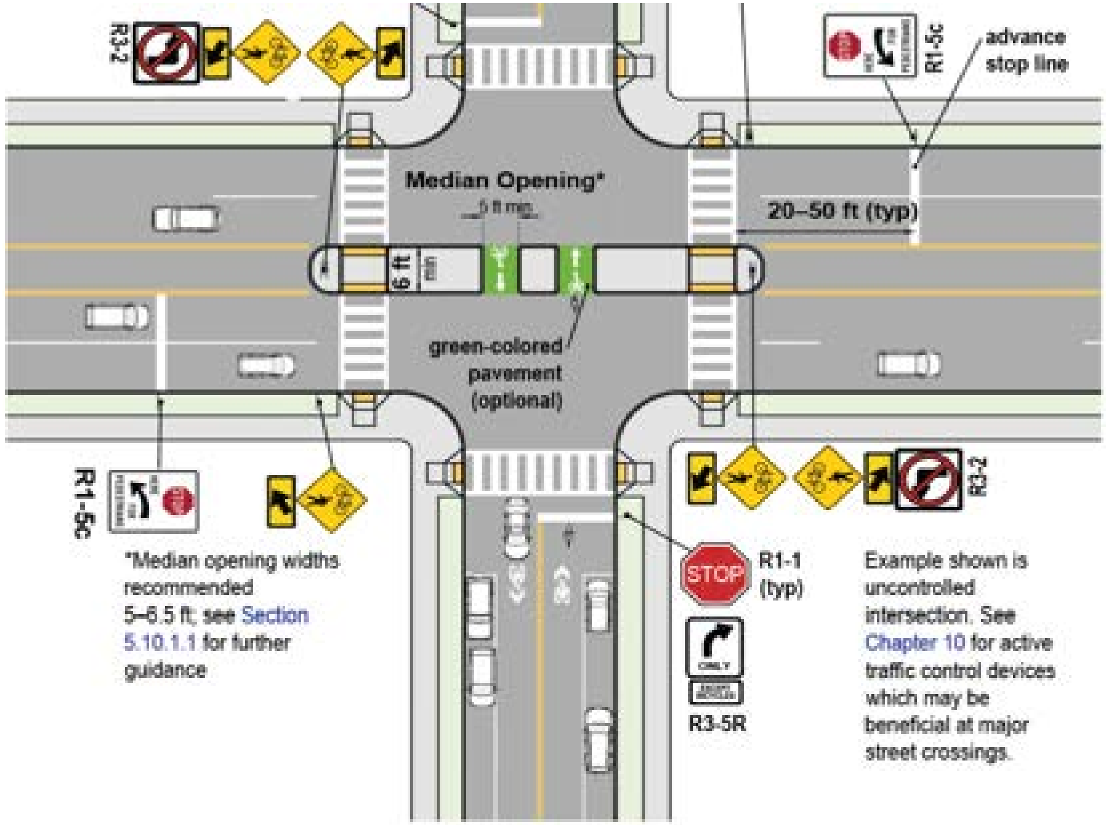


Figure 8-11: Example of a Median Used to Divert Traffic at a Major Street Crossing

TOOLE DESIGN

8.7. Traffic Controls for Major Street Crossings

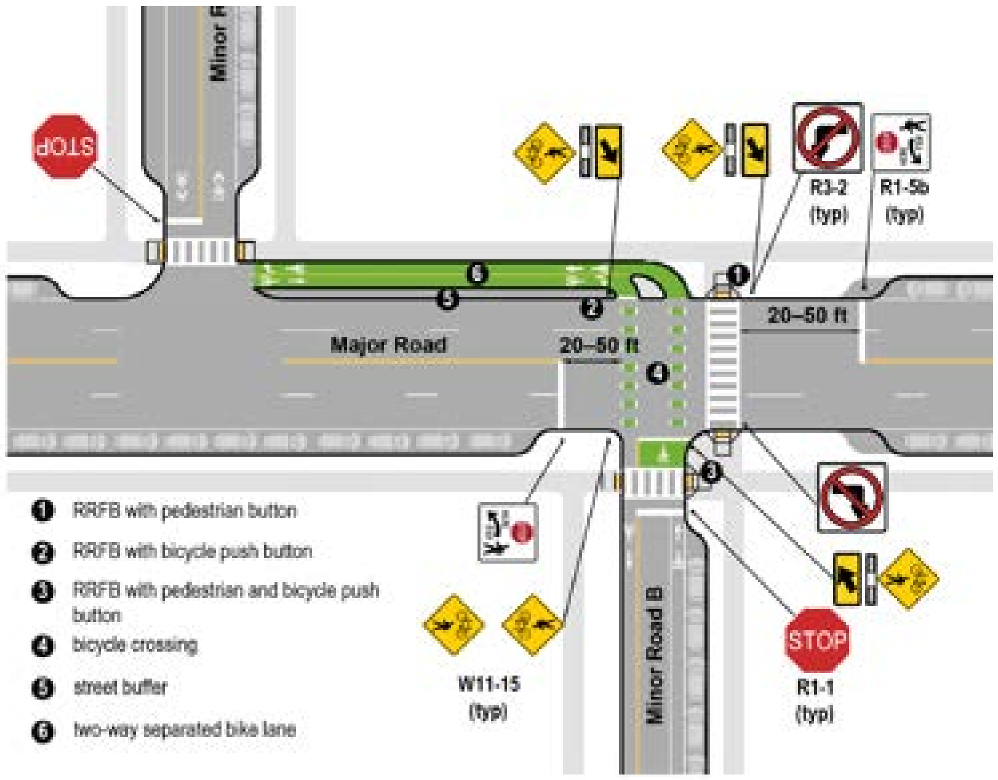


Figure 8-15: Example of Connecting Offset Bicycle Boulevard Segments Using a Two-Way Separated Bike Lane

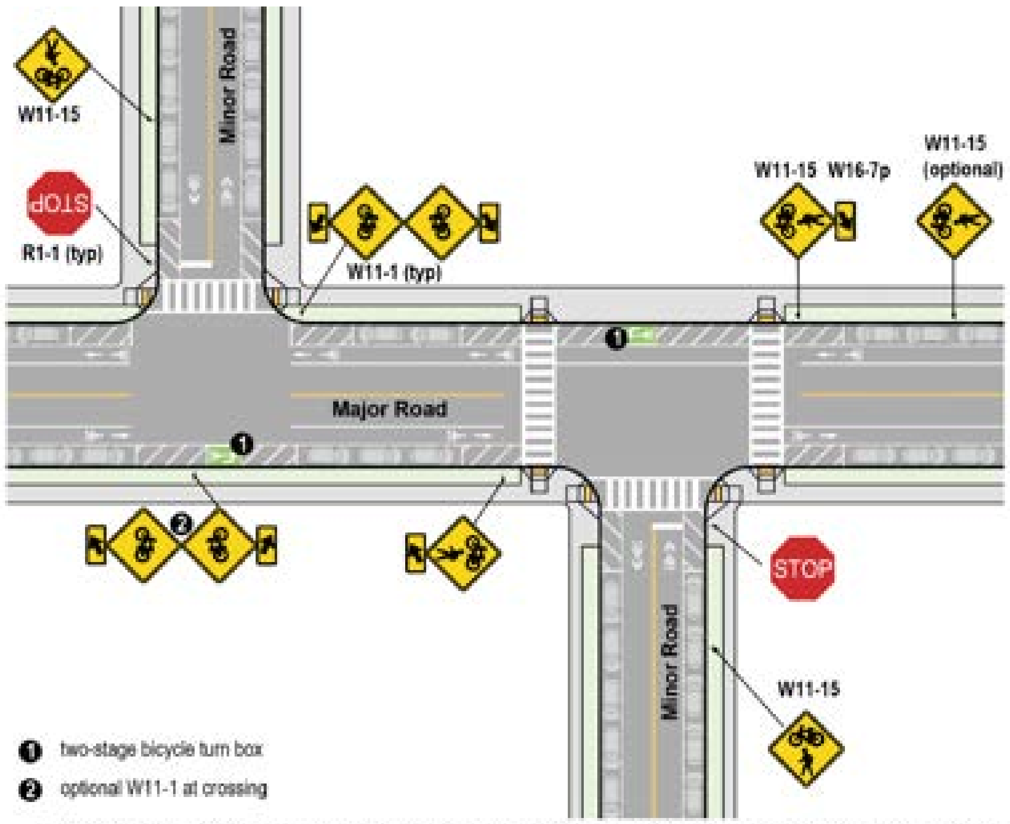


Figure 8-14: Example of Connecting Offset Bicycle Boulevard Segments Using Bike Lanes and Two-Stage Bicycle Turn Boxes

TOOLE DESIGN

Chapter 9 – Shared Lanes and Bicycle Lanes

9.1 Introduction

9.2 Design User Profile Considerations

9.3 Shared Lanes and Shared Roadways

9.4 Bicycle Lane Considerations

9.5 Buffered Bicycle Lanes

9.6 Bicycle Lane Considerations Adjacent To Parking and Loading

- 9.7 Bicycle Lane Considerations at Bus Stops
- 9.8 Advisory Bicycle Lanes (Experimental)
- 9.9 Bicycle Lanes on One-Way Streets
- 9.10 Bicycle Lanes on One Side of Two-Way Streets
- 9.11 Counterflow Bicycle Lanes
- 9.12 Bicycle Lanes at Intersections, Driveways, and Alleys

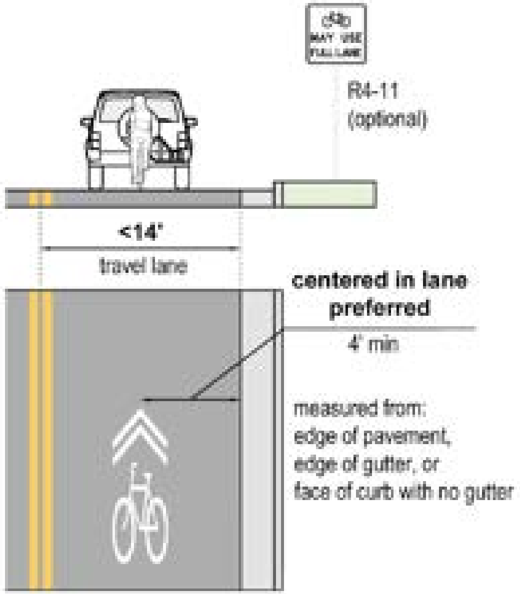
9.3.2. Limited Effectiveness of Wide Outside Lanes

Figure 9-1: Shared Lane Conditions (Rural Context, Suburban Context, Urban Context)

Rural Roadway

Suburban Arterial

Figure 9-3: Shared Lane Marking Lateral Placement in Travel Lanes < 14 Feet Without Parking



TOOLE DESIGN

9.4.1. Bicycle Lane Widths

Table 9-1: One-Way Standard Bicycle Lane Widths

One-Way Standard Bike Lane Widths				
Bike Lane Context	Practical Minimum (ft)	Recommended Lower Limit (ft)	Recommended Upper Limit (ft)	Practical Maximum (ft)
Adjacent to edge of Pavement	4[1]	5	7	8[3]
Adjacent to curb (exclusive of gutter)	5[1]	6	7	8[3]
Between through lanes and turn lanes[2]	5[1]	6	7	8[3]
Between buffers	4	5	7	8[3]
Adjacent to parking	5	6	7	8[3]
To allow occasional passing or side-by-side bicycling[4]	6.5	8[3]	10[3]	11[3]

Notes

[1]Shoulders should be provided in lieu of narrow bicycle lanes to avoid confusion below the practical minimum width.

[2]Buffers are desirable where bicycle lanes are located between through lanes and turn lanes, especially as motorist speeds exceed 30 mph.

[3]Buffered bike lanes or separated bike lanes should be considered in lieu of wider bicycle lanes to avoid confusion with a parking or travel lane.

[4]A minimum of 6.5 ft is necessary for occasional passing and 8 ft or more for comfortable side-by-side bicycling.

9.5. Buffered Bicycle Lanes

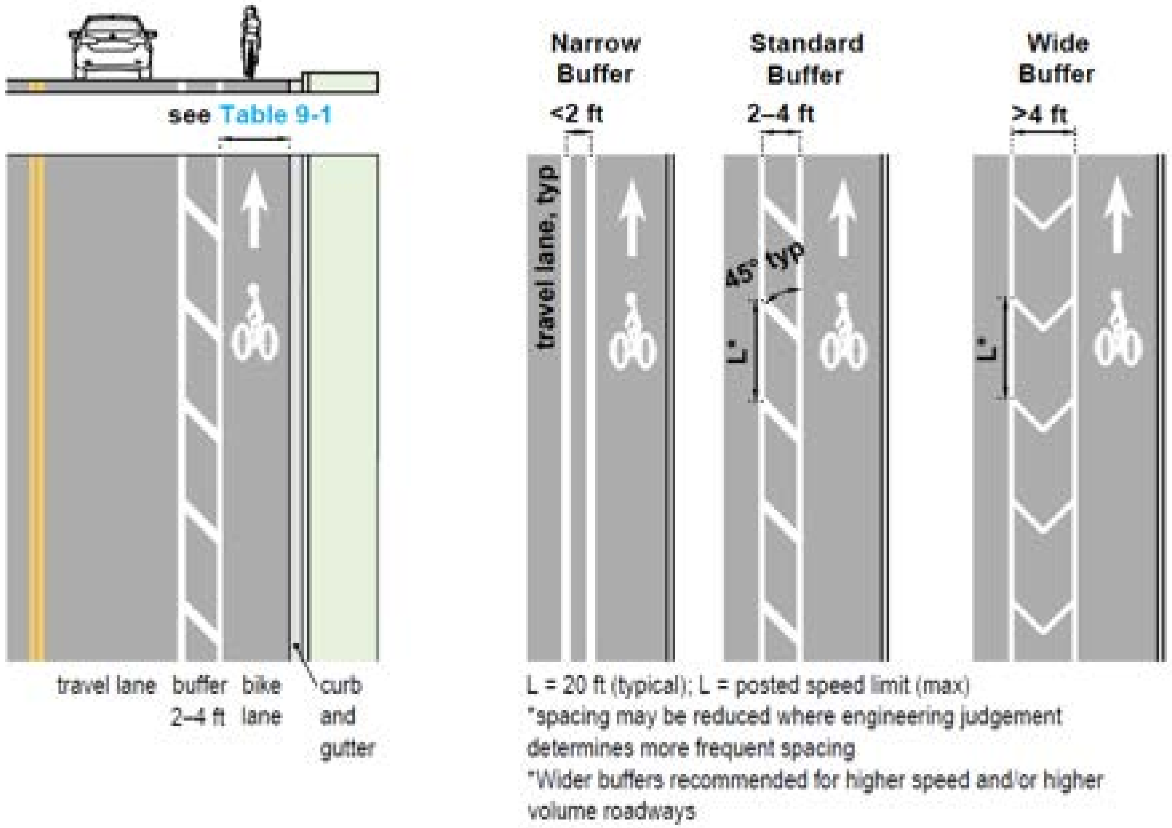


Figure 9-9: Buffer Design Options

TOOLE DESIGN

9.6.4. Bicycle Lanes Adjacent to Parallel Parking and Loading

9.6.4.1 Minimum Width Bike Lane Considerations

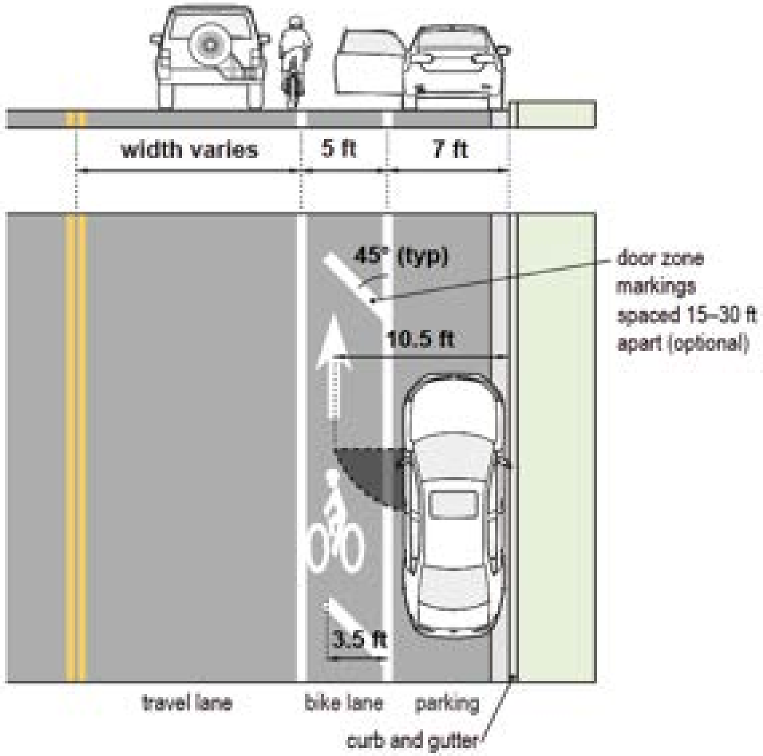


Figure 9-10: Constrained Bike Lane Adjacent to Parking Example

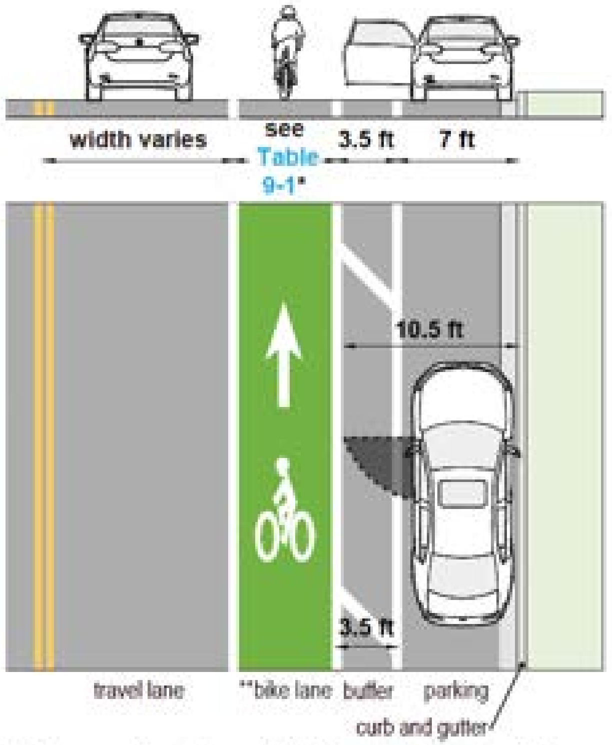


Figure 9-11: Bike Lane with a Door Zone Buffer adjacent to Parking

TOOLE DESIGN

9.12.3. Right Turn Lane Considerations

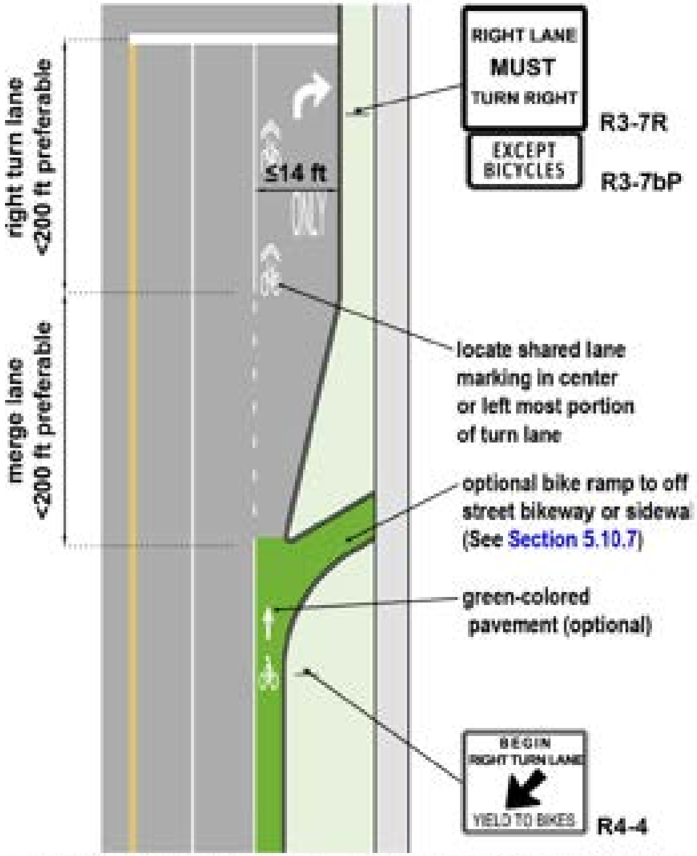


Figure 9-22: Example Right-Turn Only Lane with Shared Lane Markings

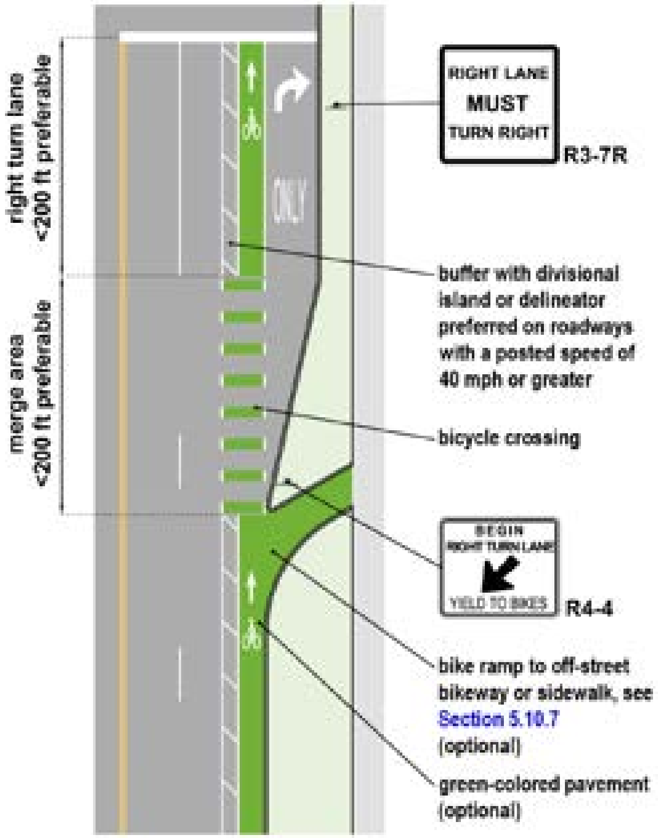


Figure 9-24: Example Bike Lanes on Streets >40 mph or Right-Turn Lanes >200 ft

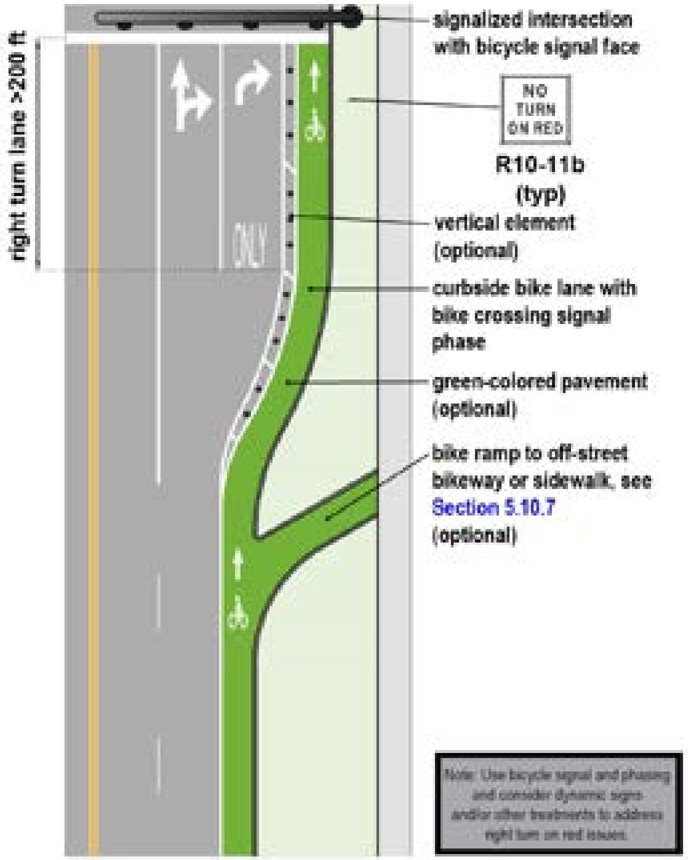


Figure 9-26: Example Bike Lane Approach to a Through-Right and a Right-Turn Only Lane

TOOLE
DESIGN

Chapter 10 – Traffic Signals and Pedestrian Hybrid Beacons

10.1 Introduction

10.2 Design Guidance for Traffic Signal Control

10.3 Traffic Signal Phasing for Managing or Reducing Conflicts

10.4 Traffic Signal Timing for Bicyclists

10.5 Bicycle Signal Design Consideration

10.6 Detection for Bicycles

10.7 Design Guidance for Pedestrian Hybrid Beacons

10.8 Toucan Crossings with Traffic Signals

10.2.4. Traffic Signal Indication Options for Bicyclists

Bike signal head warrant:

- Leading or protected phasing
- Contra-flow movements
- Signal heads beyond cone of vision

Bike signal head application:

- Can only be used without conflicting vehicle turns

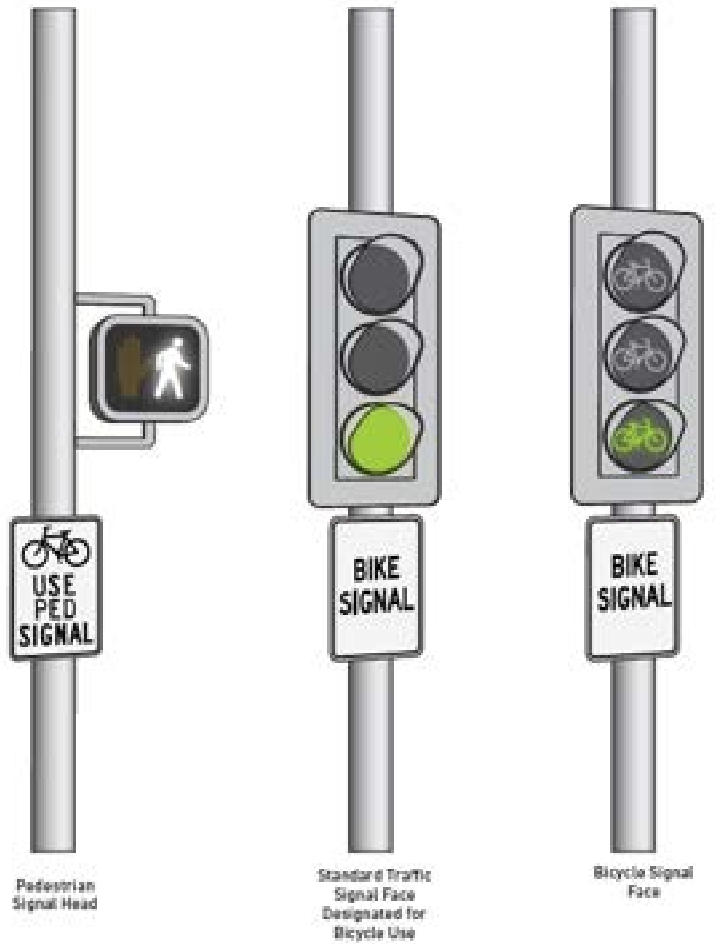
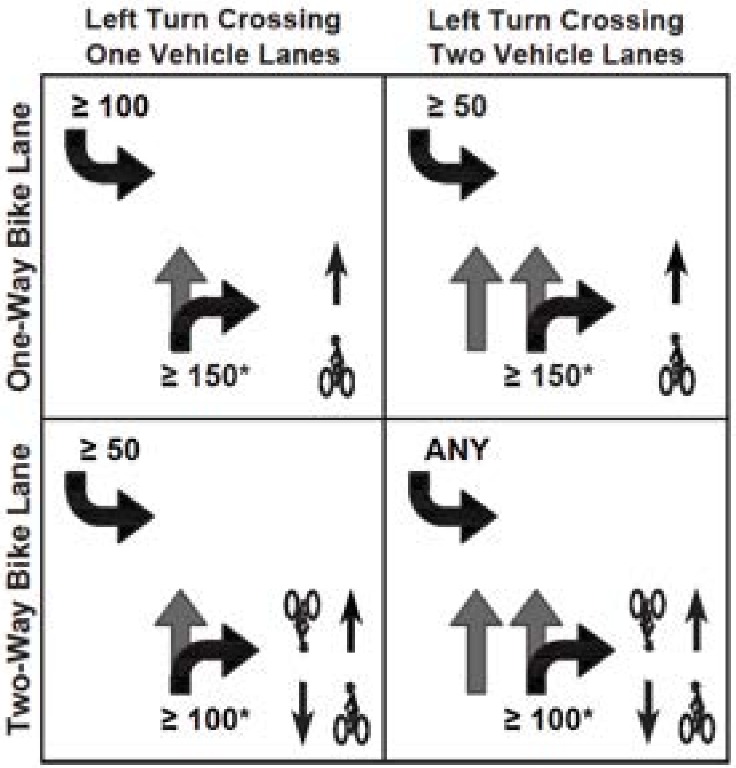


Figure 10-2: Examples of Signal Indication Options for Bicyclists

TOOLE DESIGN

10.3.5. Signal Phasing Schemes for Reducing Conflicts

Table 10-1: Recommended Hourly Turning Traffic Thresholds for Time-Separated Bicycle Movements



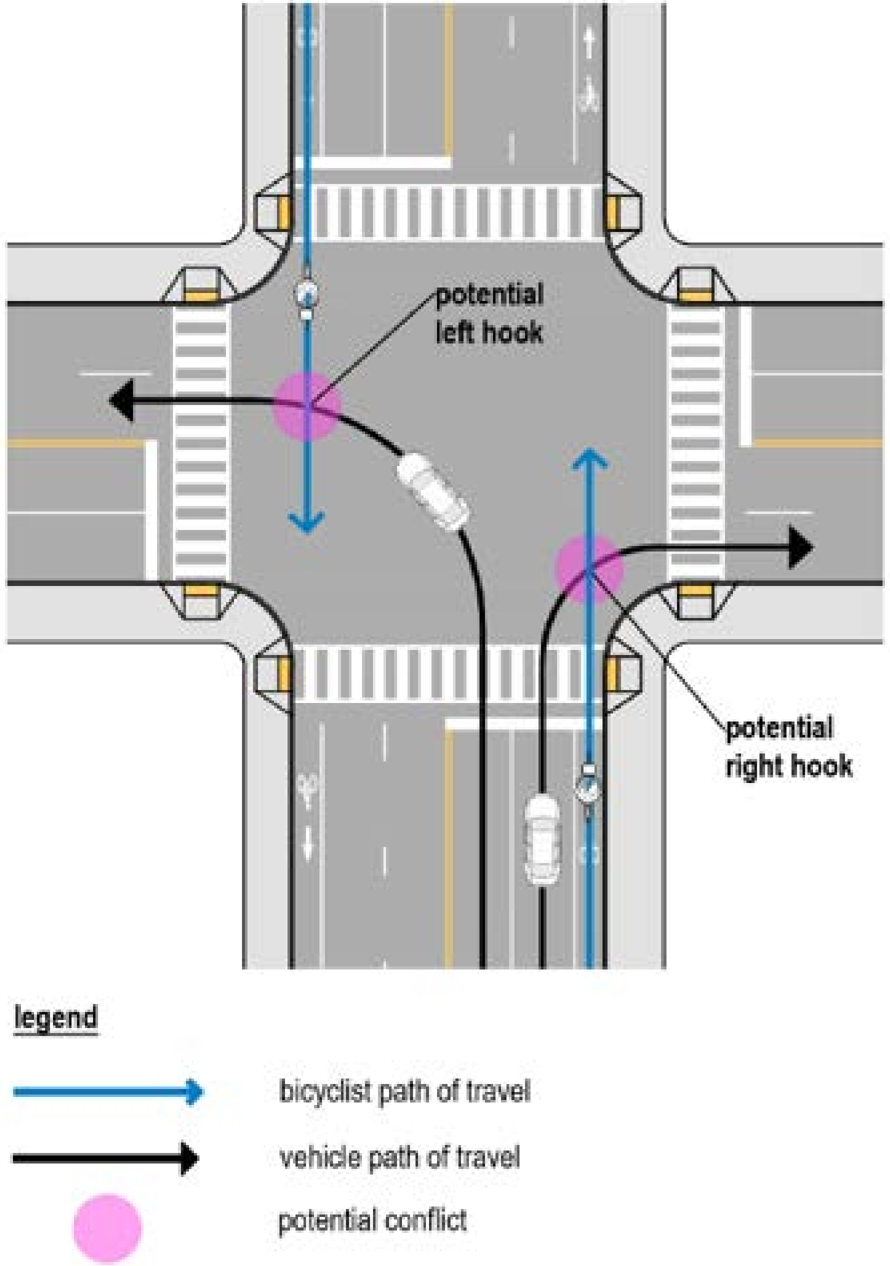


Figure 10-3: Left-Hook and Right-Hook Graphic

10.6. Detection for Bicycles
10.6.1.1 Pushbuttons for Bicyclists

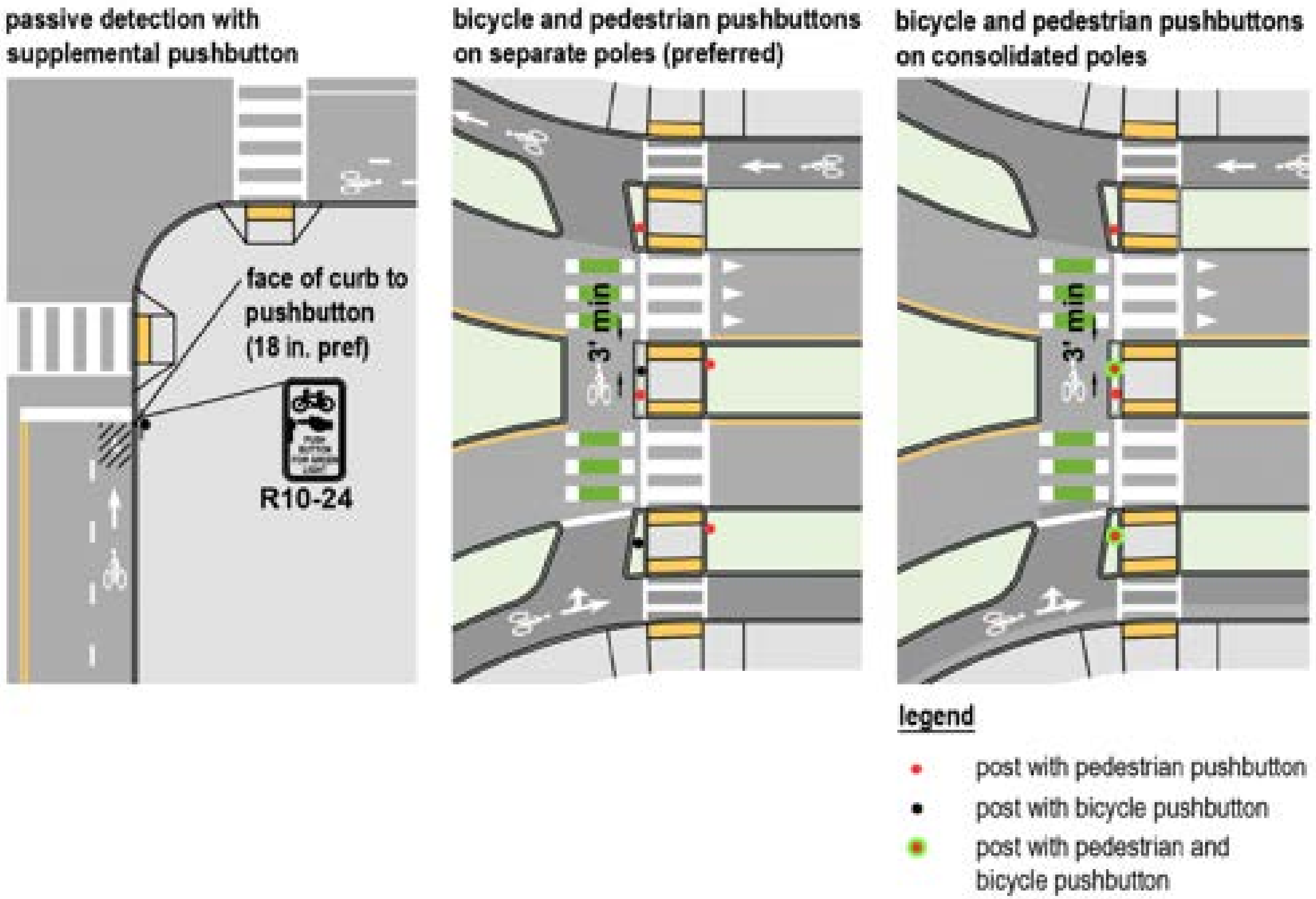


Figure 10-12: Pushbutton Locations

Figure 10-13: Example of Curbside Bicycle Pushbutton

10.4.1. Green Time, Change Interval and Clearance Intervals for Bicyclists

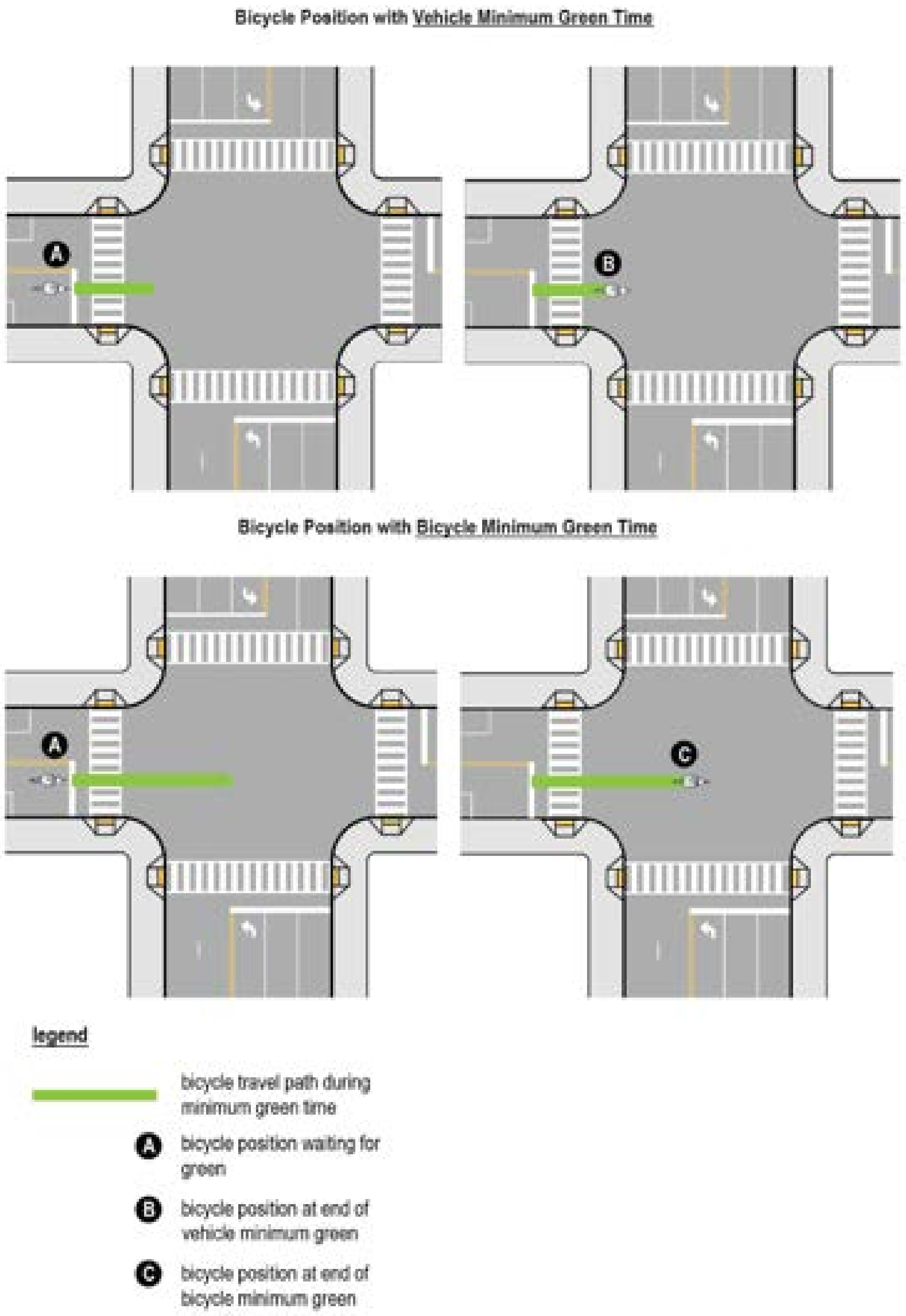
Vehicle
Minimum Green

- vs -

Bicycle
Minimum Green

Table 10-2: Bicycle Minimum Green Time Equation

Bicycle Minimum Green Time Equation		
$G_{min} = t + \frac{1.47v}{2a} + \frac{d+L}{1.47v}$		
Where:		
G_{min}	=	bicycle minimum green time (s)
v	=	attained bicycle crossing speed (assumed 8 mph)
t	=	perception reaction time (generally 1.5 s)
a	=	bicycle acceleration (assumed 2.5 ft/s²)
d	=	distance from stop bar to middle of the intersection (ft)
L	=	typical length of a bicycle (6 ft)



TOOLE DESIGN

Chapter 11: Bicycle Facility Design at Interchanges, Alternative Intersections, and Roundabouts

11.1 Introduction

11.2 Basic Design Principles

11.3 Exit and Entrance Ramps

11.4 Multiple-Threat Conditions

11.5 Motorist Left Turns

11.6 Designs that Place Bicyclists in Constrained Areas

11.7 Conflicts between Bicyclists and Pedestrians in Shares Spaces

11.8 Channelized Right-Turn Lanes

11.9 Alternative Intersection Design Considerations

11.10 Roundabouts

11.3. Exit and Entrance Ramps

- On-road and off-road options
- Bike ramp to access to sidewalk
- Sidewalk becomes shared use path
- Perpendicular crossings

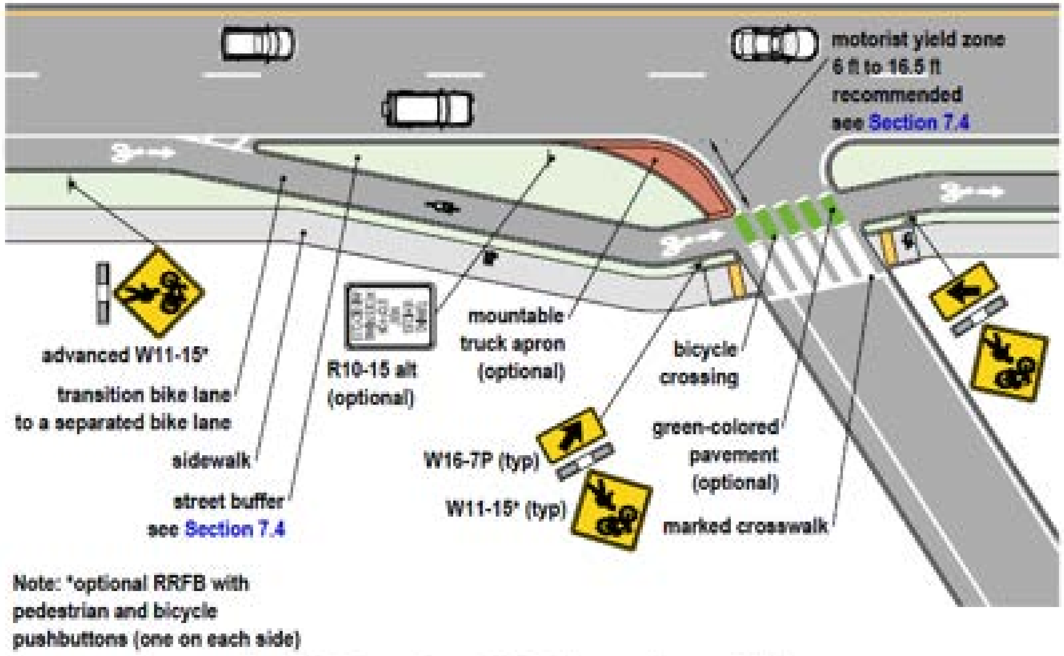


Figure 11-4: Entrance Ramp with Truck Apron and Separated Bike Lane

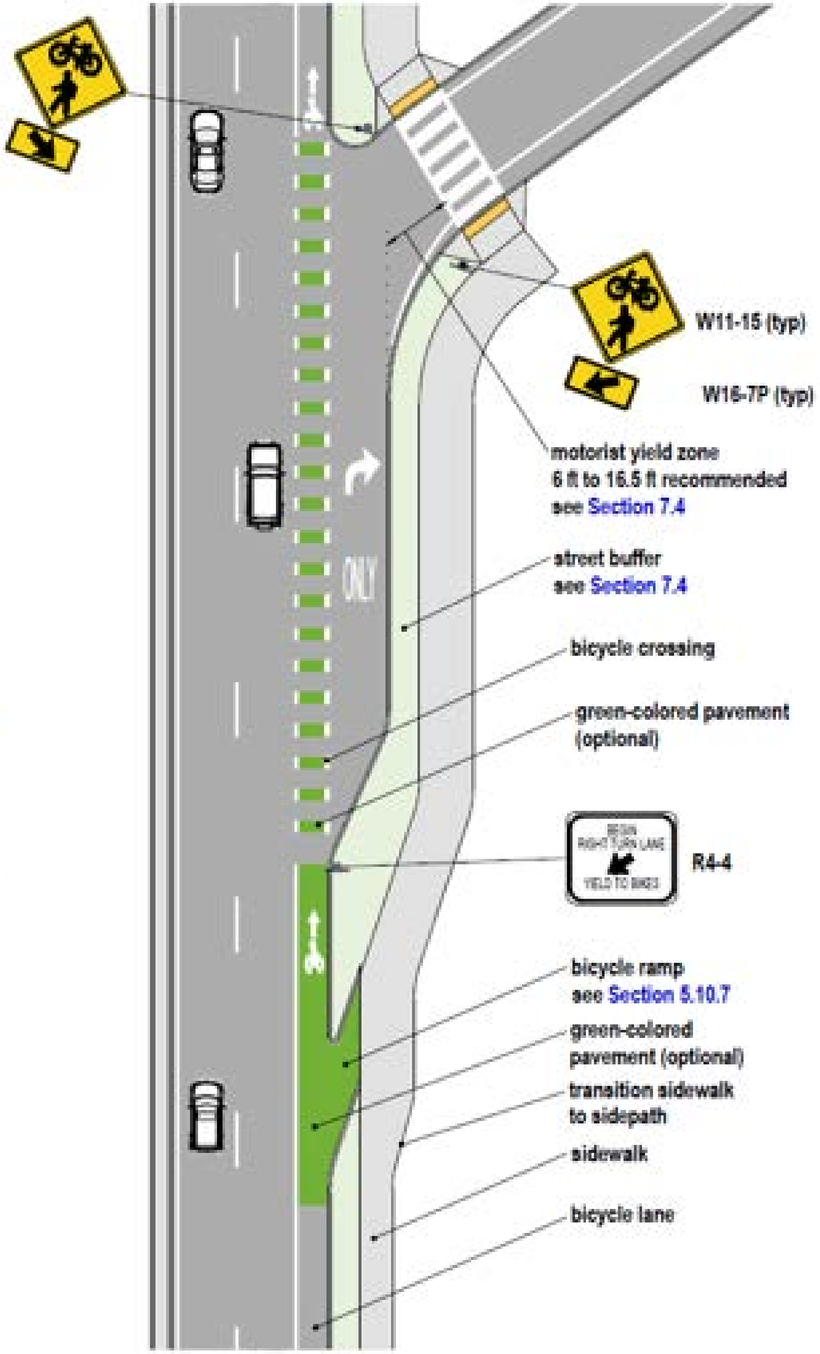


Figure 11-5: Entrance Ramp with Right-Turn Lane, Bike Lane, and Side Path

TOOLE DESIGN

11.3.3. Merging and Weaving Areas

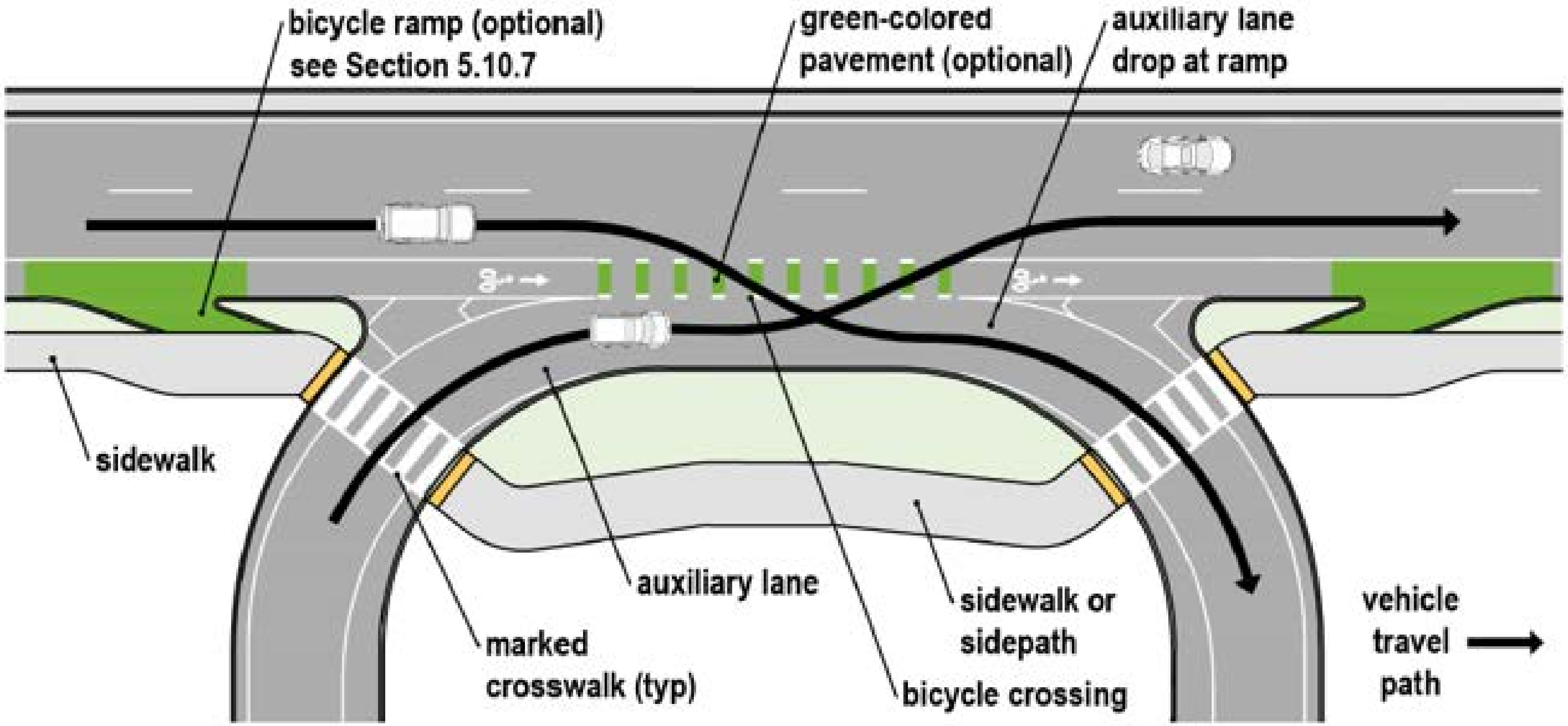


Figure 11-9: Bike Lane Positioned in High-Exposure Weaving Area

TOOLE DESIGN

11.7. Conflicts between Bicyclists and Pedestrians in Shared Space

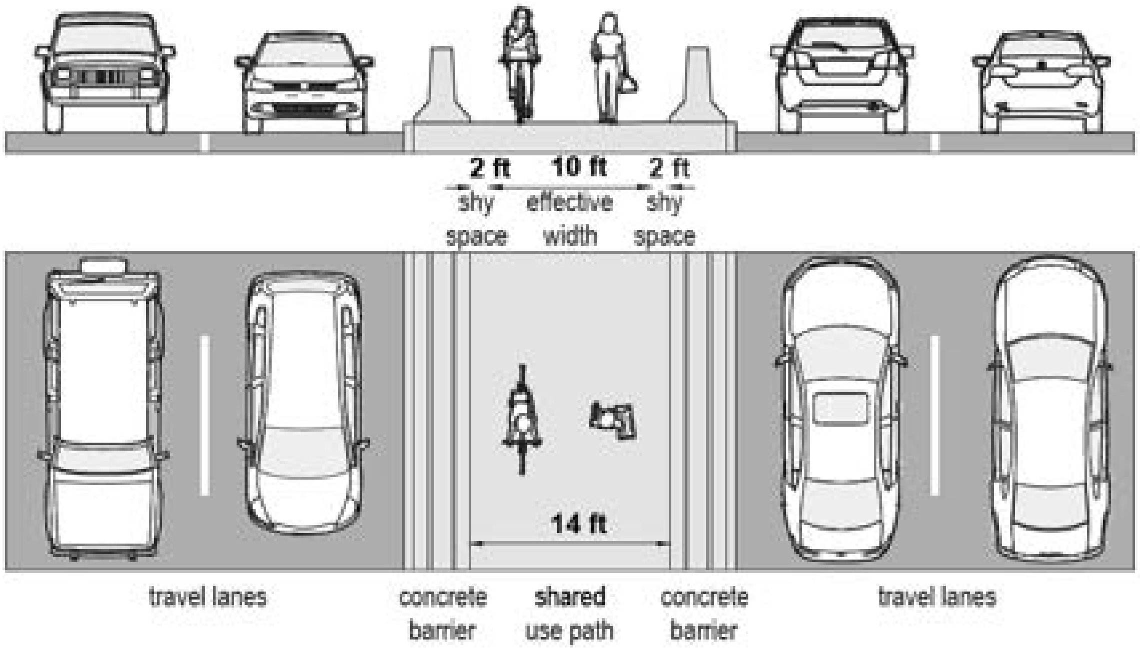


Figure 11-11: Constrained Median Shared Use Path (10 ft wide) with Concrete Barrier Buffers

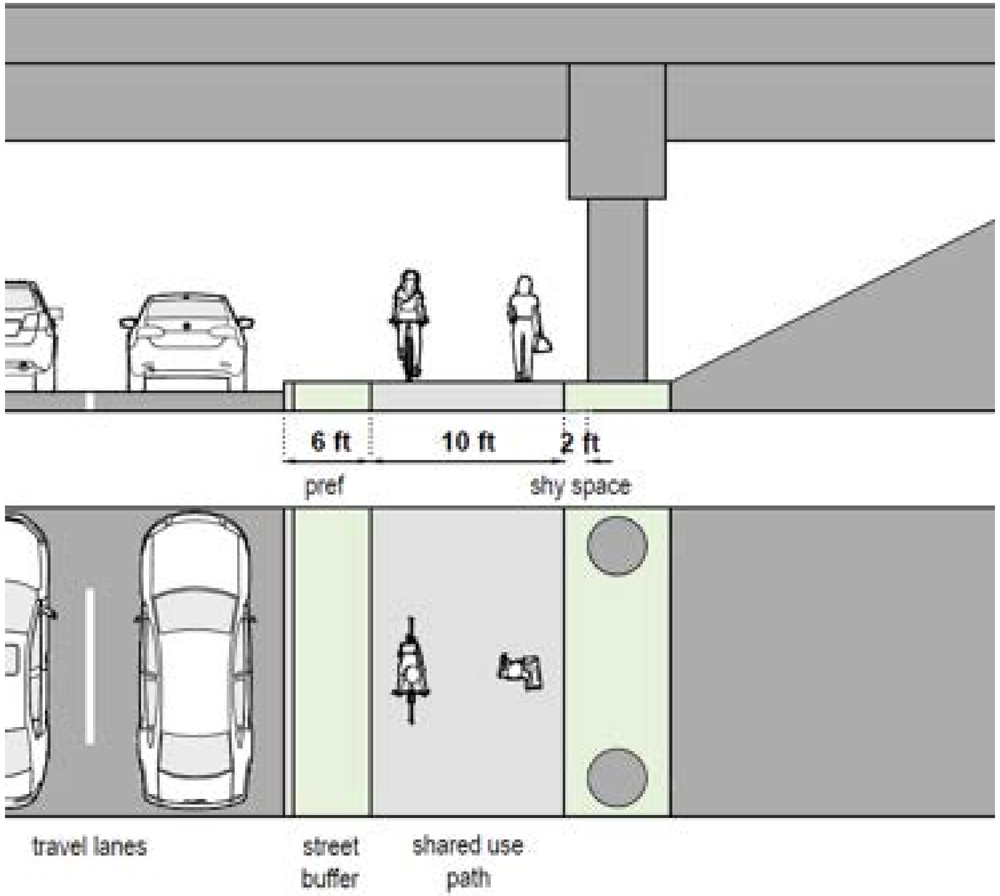


Figure 11-12: Side Path between Travel Lanes and Bridge Piers with Preferred Buffers

TOOLE DESIGN

11.8. Channelized Right-Turn Lanes

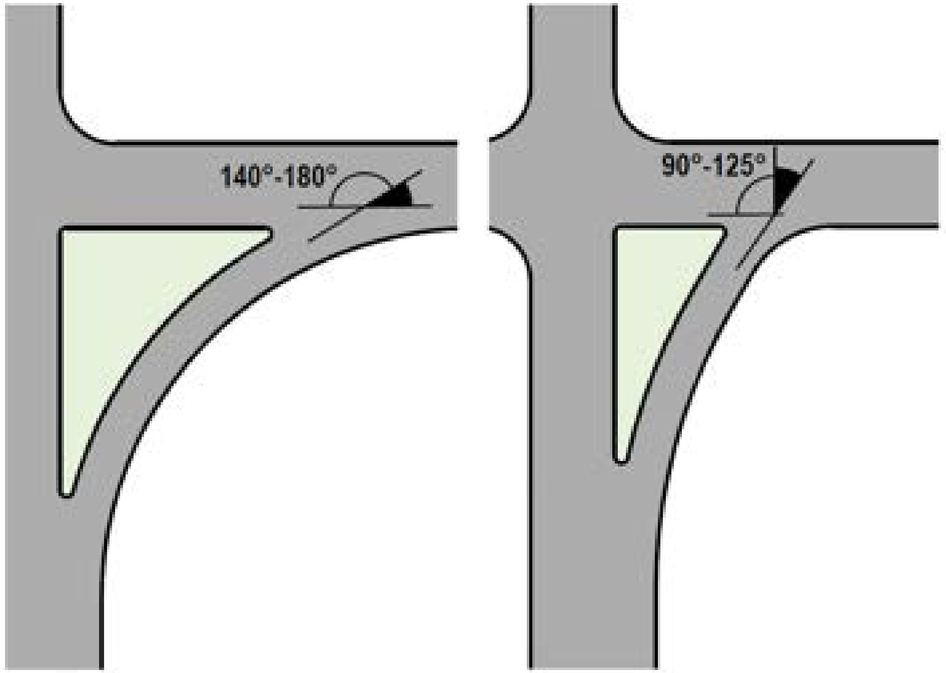


Figure 11-13: Channelized Right-Turn Lane Approach Angles

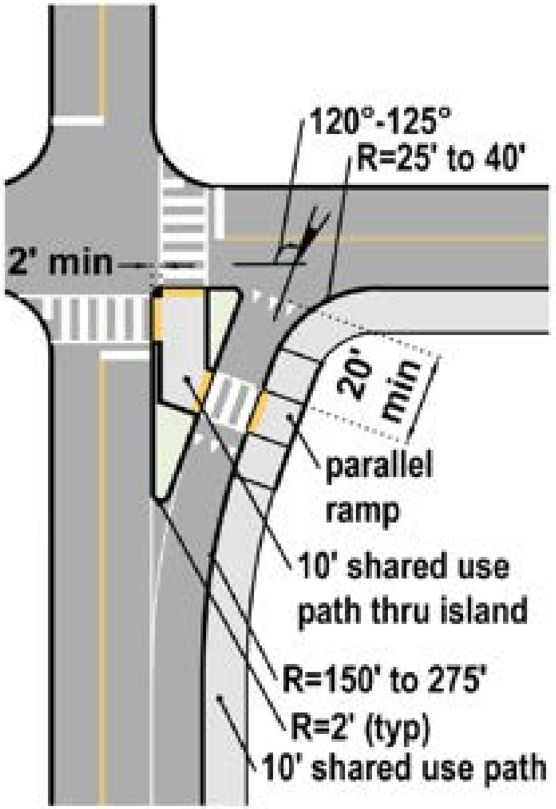


Figure 11-14: Channelized Right-Turn Refuge Island

11.10. Roundabouts

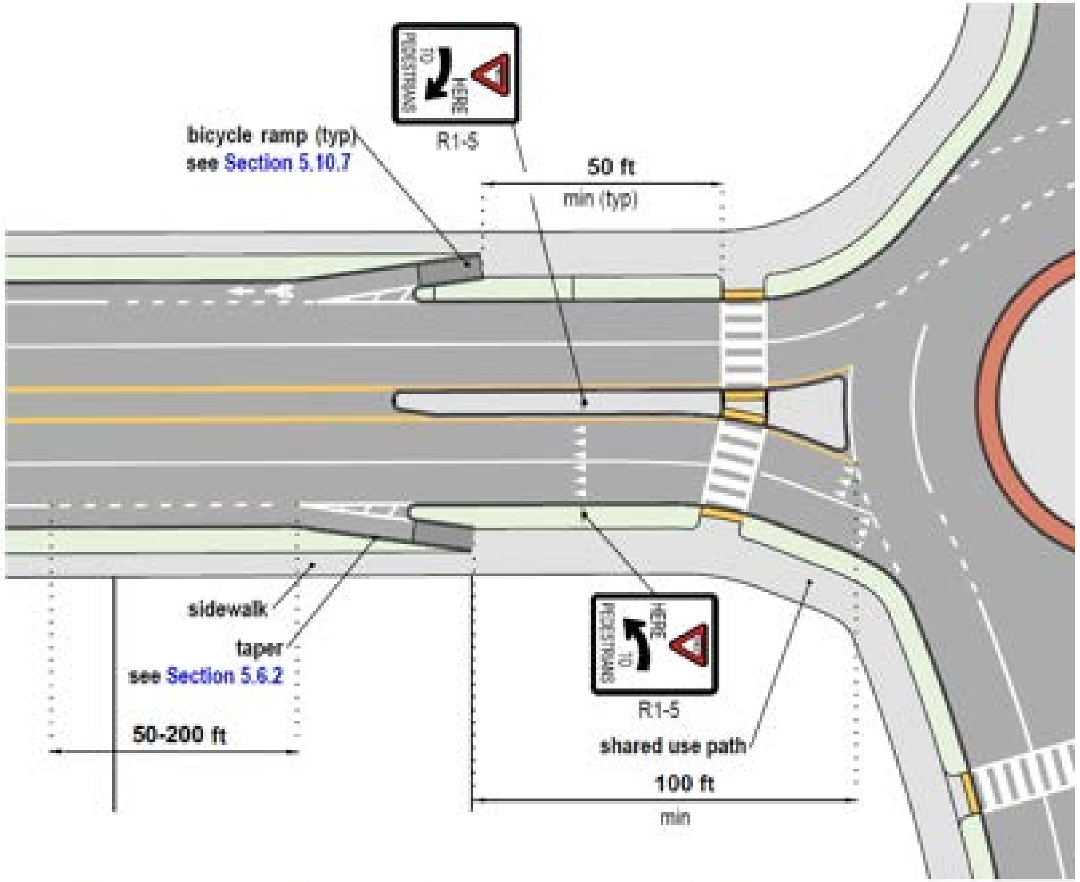


Figure 11-16: Typical Layout of Bike Lane Transitions to Shared Use Path at Multilane Roundabout with Bike Ramps

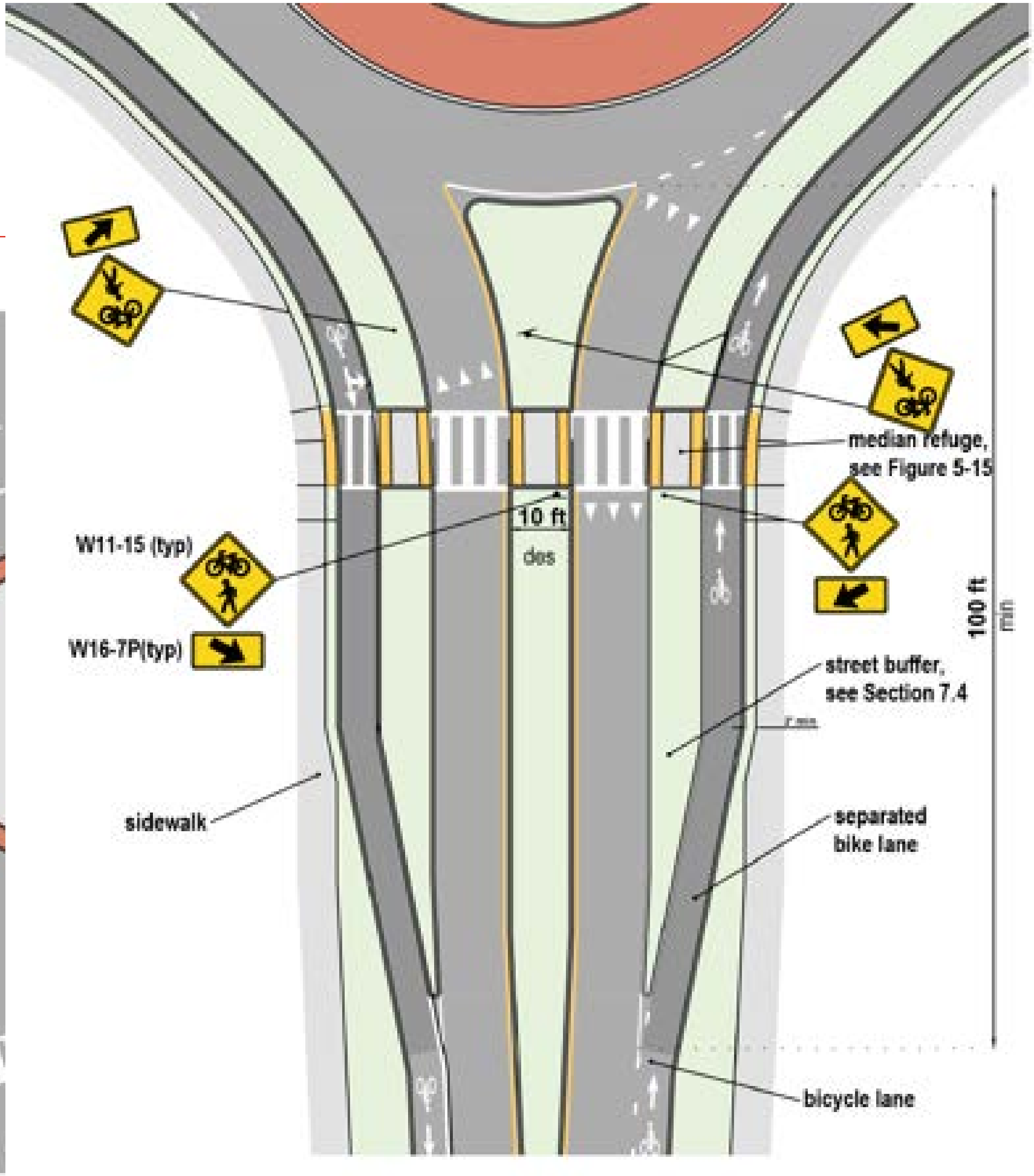


Figure 11-17: Typical Layout of Separated Bike Lanes at Roundabout

TOOLE DESIGN

Chapter 12 – Rural Area Bikeways and Roadways

12.1 Introduction

12.2 Safety Context of Rural Roads

12.3 Design User Profiles

12.4 Rural Bikeway Treatments

12.5 Pavement Surface Quality on Rural Roadways

12.6 Shared Use Paths and Sidepaths

12.7 Design Considerations for Bridges, Viaducts, and Tunnels in Rural Areas

12.8 Bicycle Travel Along Interstates, Freeways, and Limited-Access Highways

12.9 Roundabouts

Section 12.3 - Design User Profiles

Design User:

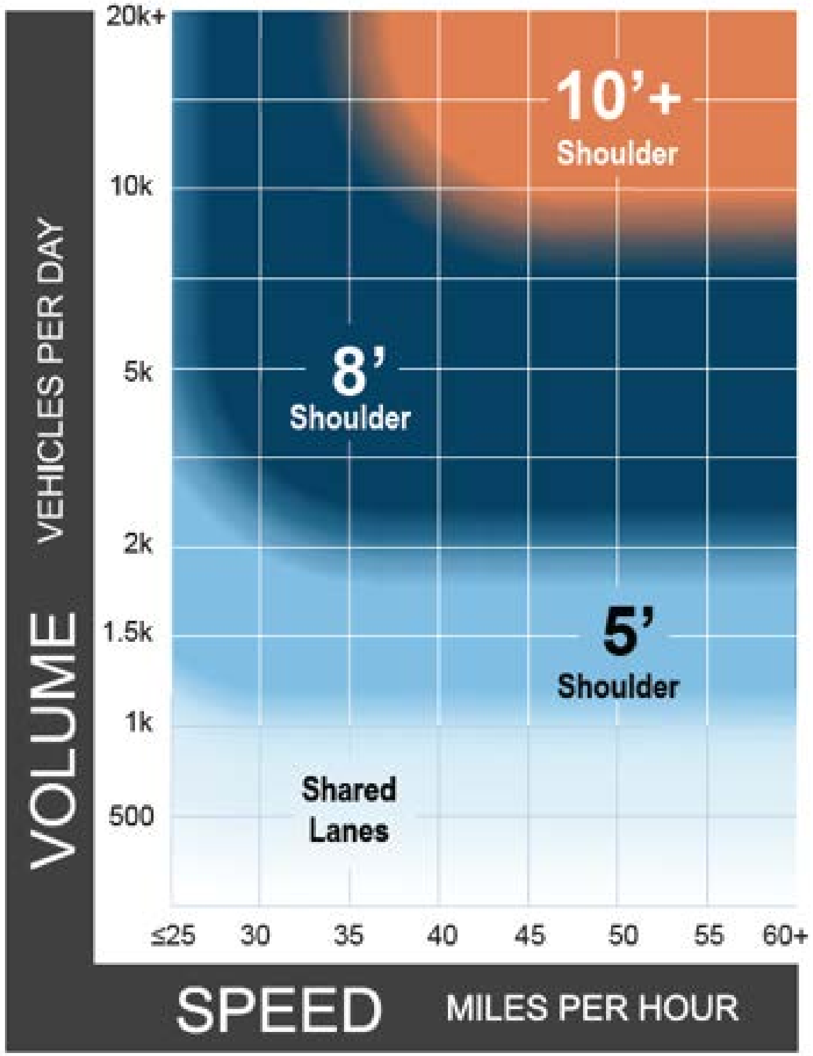
Between Towns & Villages

- ***Highly Confident***

In Towns & Villages

- ***Interested but Concerned***

Figure 4-2: Preferred Paved Shoulder Widths for Rural Roadways to Accommodate Highly Confident or Somewhat Confident Bicyclists



TOOLE DESIGN

Section 12.4.1 – Shared Roadways

TOOLE DESIGN

Section 12.4.3 Paved Shoulders

Table 12-1: Paved Shoulder Widths for Bicycling (see Chapter 12 References: FHWA, 2016b)

Paved Shoulder Widths Exclusive of Rumble Strips[i] for Bicycling				
Design Year Average Daily Traffic (ADT) and Posted Speed (MPH) Thresholds	Practical Minimum[ii]	Recommended Range		Practical Maximum
		Lower Limit[ii]	Upper Limit	
< 2,000; all speeds	2 ft	3 ft	5 ft[ii]	10 ft
2,000 - 6,000; all speeds	2 ft	4 ft	6 ft[ii]	10 ft
6,000 - 10,000; all speeds	4 ft	6 ft	8 ft[ii]	10 ft
> 10,000; ≤ 35 mph	5 ft	6 ft	8 ft[iii]	12 ft[iii]
> 10,000; > 40 mph[iv]	5 ft	6 ft	10 ft[iii]	12 ft[iii]

Notes

[i]See Section 12.5.1 for rumble strip design considerations.

[ii]Where roadside barriers, walls, or other vertical elements are present, they should be offset a minimum of 2 ft from the outer edge of the rideable shoulder to provide minimum shy distance to bicyclists (see Section 2.5.3.2.)

[iii]Where >10 percent of traffic consists of trucks.

[iv]Shared use paths are preferred.

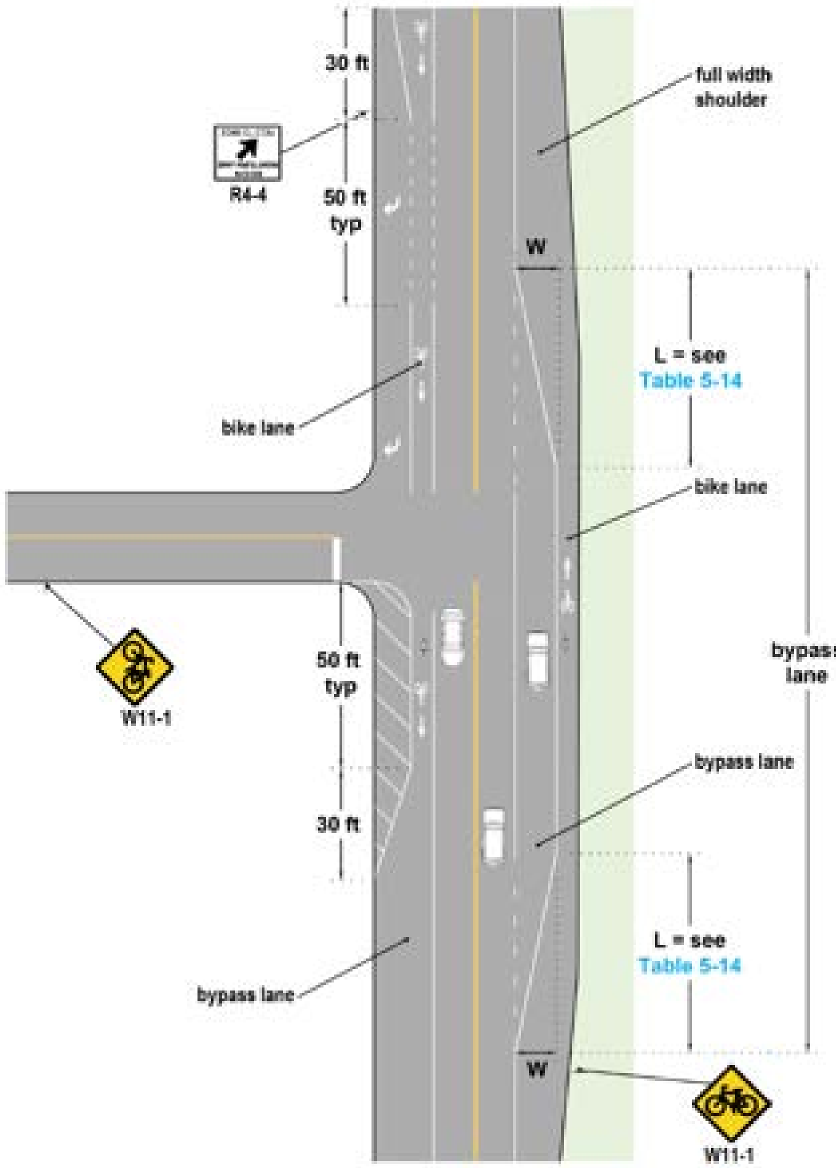
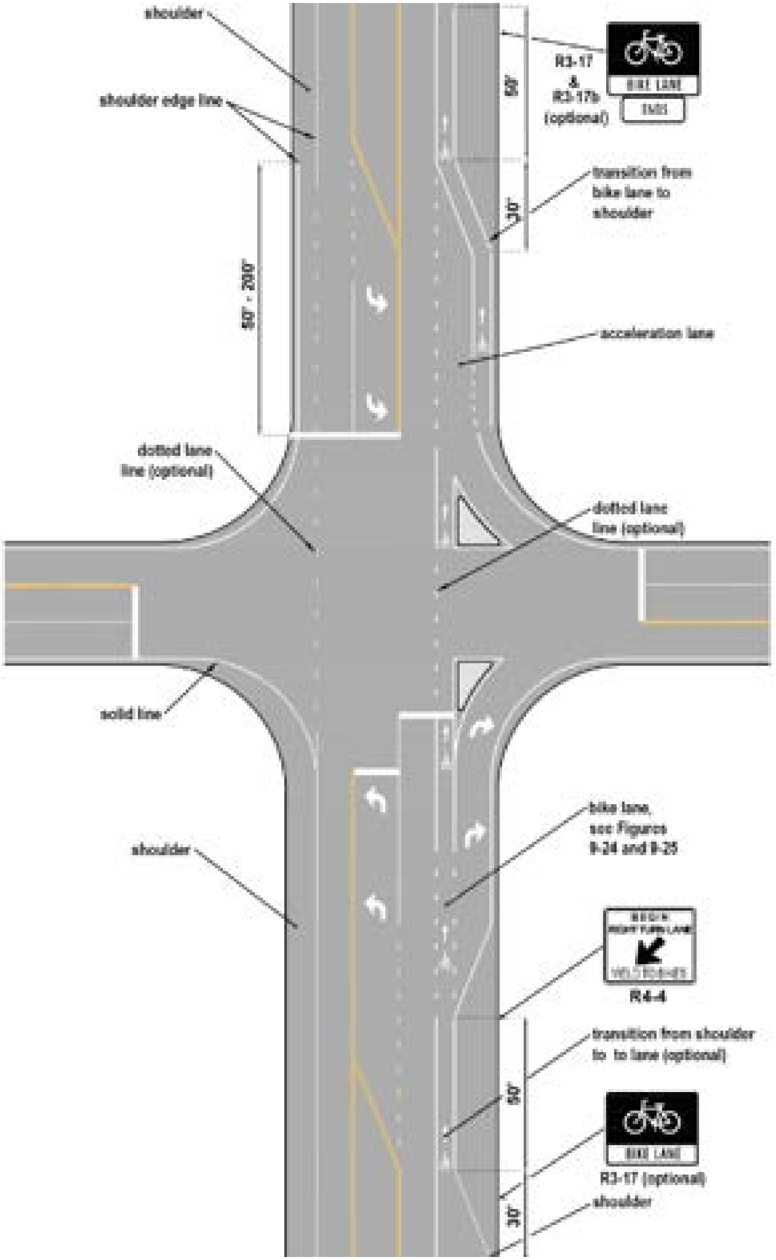


Figure 12-6: Bypass Lane with Paved Shoulder

TOOLE DESIGN

Section 12.4.3 Paved Shoulders at Intersections



TOOLE DESIGN

12.5.1. Rumble Strip Placement and Design

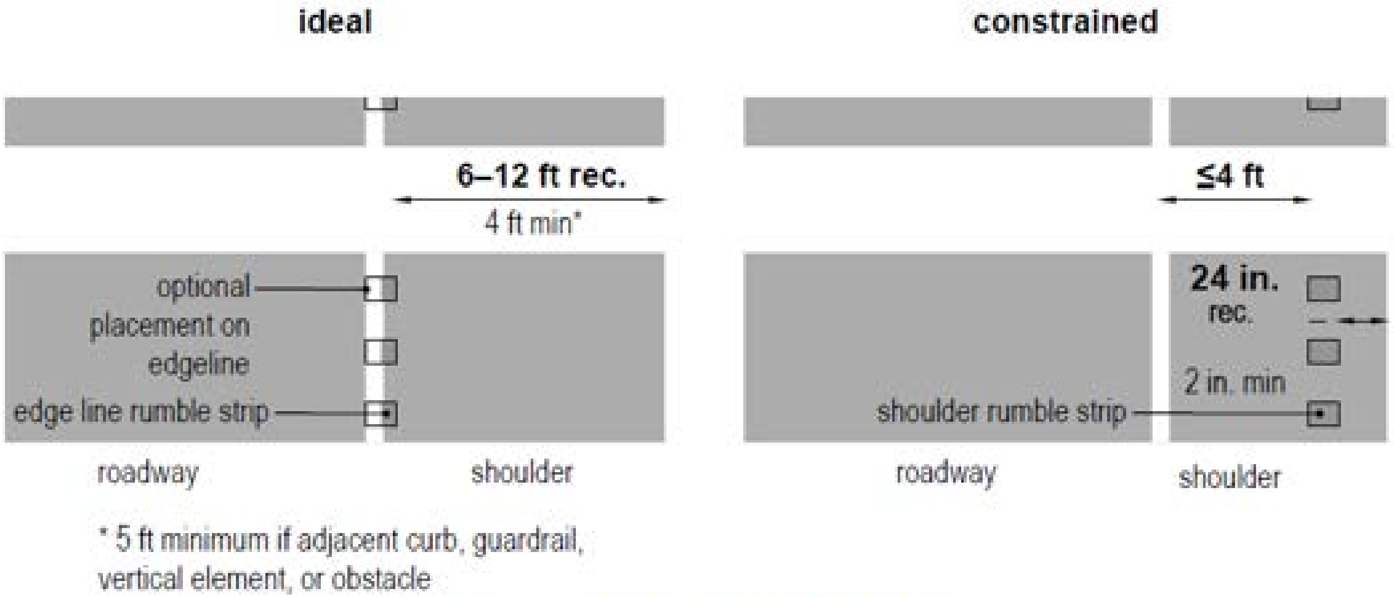
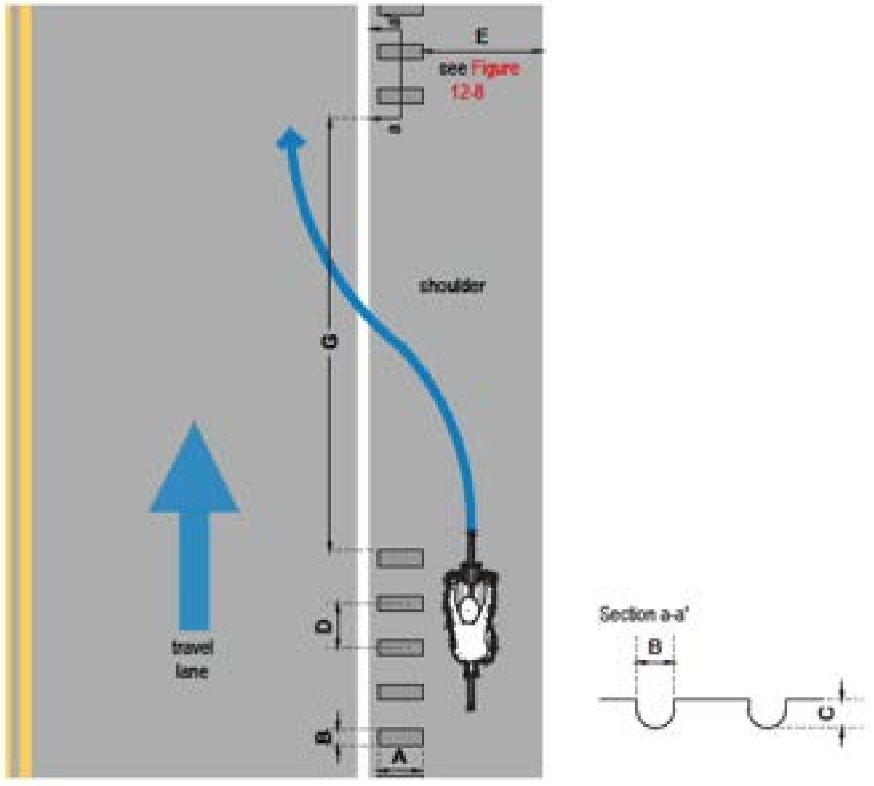


Figure 12-8: Rumble Strip Placement Options



Definitions	
Length (A)	Dimension of rumble strip measured lateral to the travel lane
Width (B)	Dimension of rumble strip measured parallel to the travel lane
Depth (C)	Vertical distance measured from top of pavement surface to bottom of a rumble strip pattern
Spacing (D)	Dimension between rumble strip patterns
Clear Path (E)	Distance from outside (for example, right) edge of rumble strip to oustide edge of paved shoulder
Gap (G)	Distance measured parallel to roadway, between groups of rumble strip patterns

*Note: Figure not to scale.

Figure 12-9: Rumble Strip Minimum Gap Illustration

TOOLE DESIGN

Chapter 13 – Structures

13.1 Introduction

13.2 General Design Principles for Structures

13.3 Design Details for Bridges

13.4 Design Details for Underpasses

13.5 Options for Retrofitting Existing Structures

13.6 Connections to Nearby Facilities

13.2. General Design Principles for Structures

Figure 13-1: Bikeway along the Interstate 90 Bridge over Lake Washington, WA

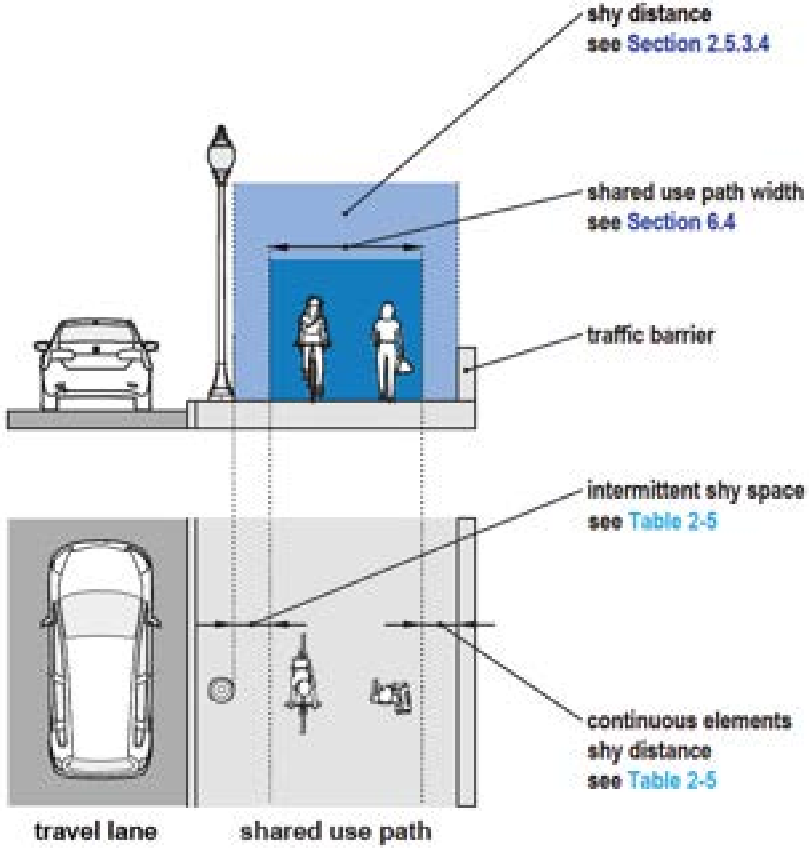


Figure 13-5: Horizontal Clearances for Shared Use Paths on Bridges Along Roads

TOOLE DESIGN

Chapter 14 – Wayfinding Systems for Bicyclists

14.1 Introduction

14.2 Core Wayfinding Approaches

14.3 When to Use Bicycle Wayfinding Signs

14.4 Design User Profile

14.5 Bicycle Wayfinding Approaches

14.6 Bicycle Wayfinding Sign Assemblies

14.7 Supplemental Information

14.8 Supplemental Wayfinding Elements

14.9 Wayfinding Sign Design: Style and Branding

14.10 Wayfinding Sign Placement and Installation

14.11 Wayfinding for Bicycle Detours and Work Zones

14.6. Bicycle Wayfinding Sign Assemblies

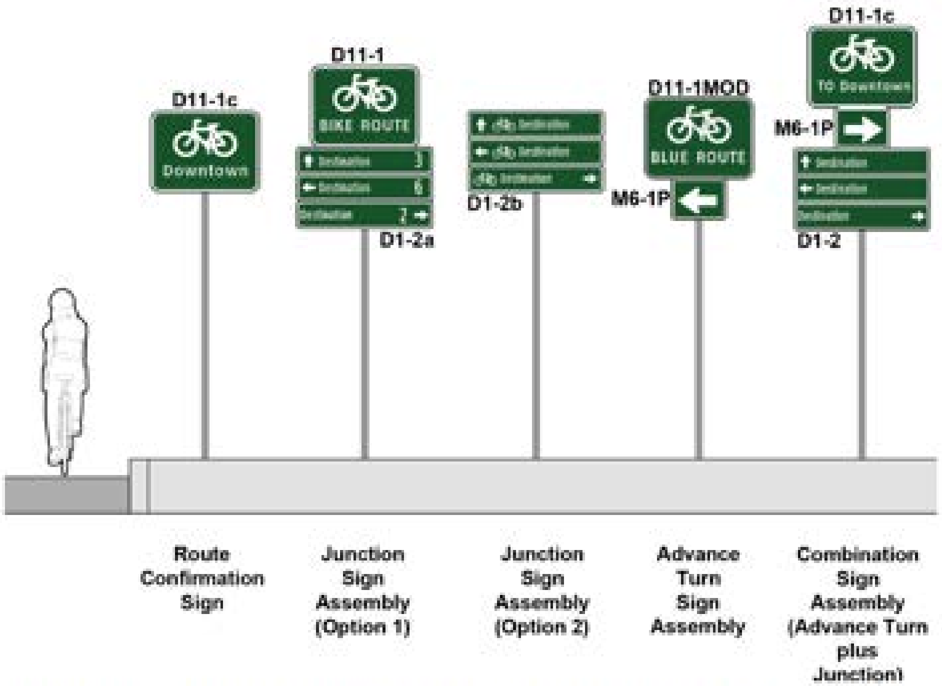


Figure 14-4: Examples of Confirmation, Decision, and Turn Sign Assemblies

MUTCD D Series

Figure 14-7: Example of Community Wayfinding

Custom

Table 14-1: Mileage Rounding Guidelines

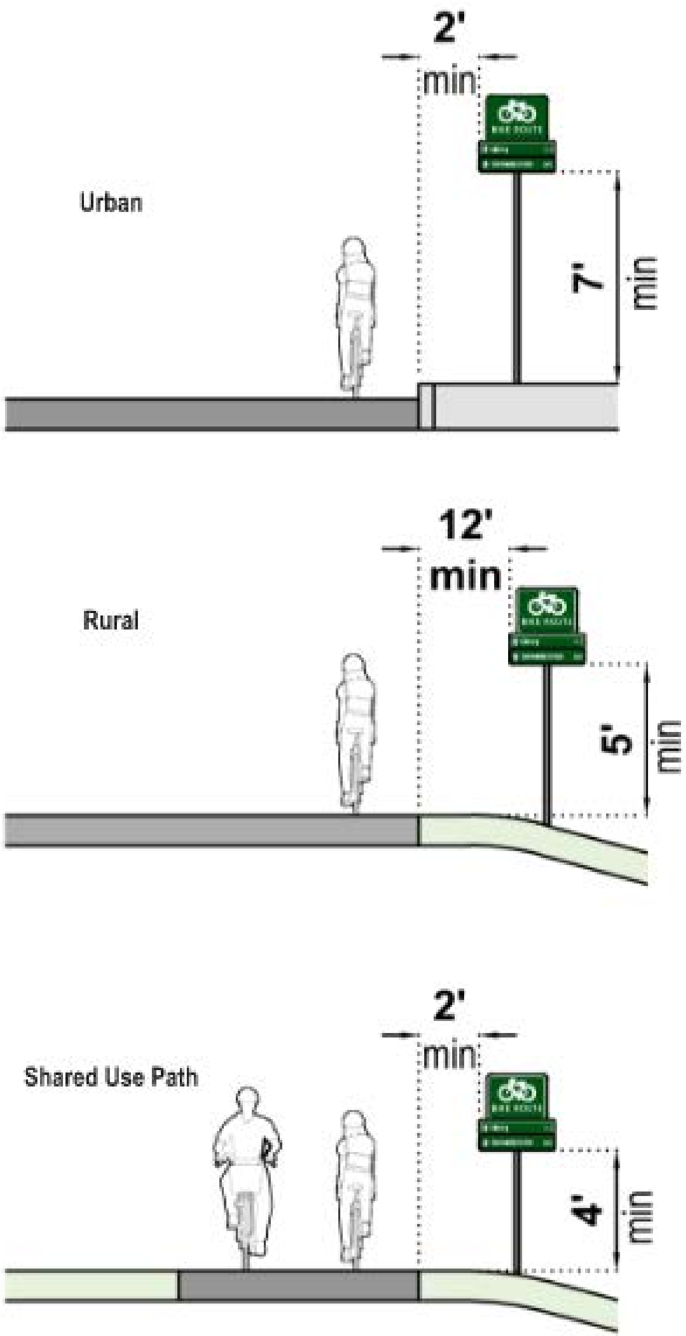
Mileage Rounding Guidelines	
Distance (mi)	Guideline
< 0.2	Do not include mileage; blocks are appropriate, if necessary
0.2 - 5.0	Round mileage to the nearest tenth of a mile
> 5.0	Round mileage to the nearest whole mile

TOOLE DESIGN

14.6. Bicycle Wayfinding Sign Assemblies

- Sign Placement and Installation
  - Vertical / horizontal clearance
  - Placement at intersections
- Wayfinding in Work Zones and Detours

Figure 14-8: Horizontal and Vertical Sign Clearances



TOOLE DESIGN

Chapter 15 – Maintenance and Operations

15.1 Introduction

15.2 Maintenance Policy and Programs

15.3 Designing for Ease of Maintenance

15.4 Maintenance Activities

15.5 Temporary Traffic Control for Bicyclists (Maintenance of Traffic)

15.2. Maintenance Policy and Programs

Figure 15-1: Examples of Debris, Faded Markings, and Snow Clearing

Figure 15-4: Fog Sealing a Shared Use Path

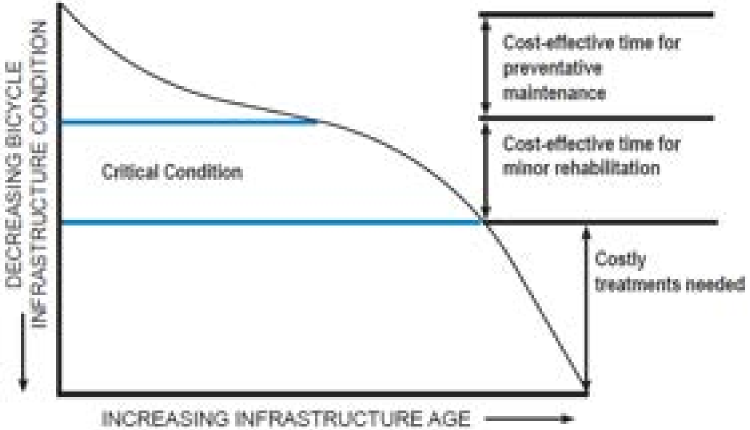


Figure 15-2: Bicycle Infrastructure Life Cycle

Table 15-1: Maintenance Equipment Types

Maintenance Equipment Types				
Type of Equipment	Corresponding Design Vehicle[a]	Width (ft)[b]	Height (ft)	Uses
3-Axle Single Unit Truck	SU-40	8	11-13	highway snow plowing, heavy construction, emergency vehicles
2-Axle Single Unit Truck	SU-30	8	11-13	ambulance, snow plowing, construction, routine maintenance
Pickup Truck	N/A	6-8	6-7	snow plowing, routine maintenance, law enforcement
Typical Skid-Steer Loader	N/A	5.5	6.5	snow plowing, routine maintenance, sweeping
Specialty Equipment	N/A	Varies by manufacturer		Varies

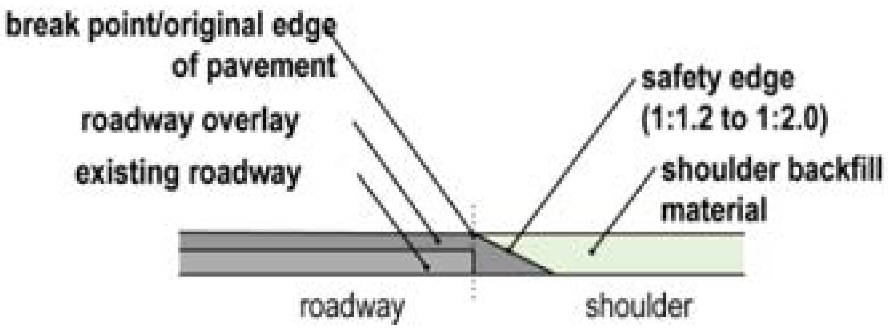
[a] For detailed information on vehicle geometry and turning radius, refer to Chapter 2 of AASHTO's *A Policy on Geometric Design of Highways and Streets* (See Chapter 15 References: AASHTO, 2018).
[b] Width of attachments such as sweeper brooms or snowplow blades may exceed the width of the vehicle.

TOOLE DESIGN

15.4. Maintenance Activities - Surfaces

- Surface Repairs
- Pavement Overlays
- Maintenance of concrete surfaces

Figure 15-6: Recommended Safety Edge$_{SM}$ configuration for AC pavements and overlays



TOOLE DESIGN

15.4. Maintenance Activities

- Clearing and Sweeping
- Vegetation
- Drainage Structures
- Signing and Pavement Markings

Table 15-2: Types of Pavement Marking Materials

Comparison of Pavement Marking Materials			
Material	Initial Relative Cost ($) = Low ($)($)($)($) = High	Lifespan (months)	Retroreflectivity ○ = Low ○○○ = High
Paint	($)	3 - 24	○
Epoxy Paint	($)($)	24 - 48	○○
Thermoplastic (sprayed)	($)($)($)	48 - 72*	○○
Preformed Tape	($)($)($)($)	36 - 96*	○○○

Note: Estimates based on 2014 comparative costs.
** Thermoplastic and tape have shorter lifespans in snowy areas where they are often damaged by snowplows, unless they are ground-in (recessed) below the pavement surface. In low-volume environments without snowplowing, markings may last substantially longer.*

Chapter 16 – Bicycle Parking, Bike Share Siting, and End of Trip Facilities

MUTCD D4-3

Figure 16-1: Directional Signage for Bicycle Parking

16.1 Introduction

16.2 Planning for Bicycle Parking

16.3 Short-Term Parking

16.4 Long-Term Parking

16.5 Rack Design

16.6 Short-Term and Long-Term Bicycle Parking Site Design

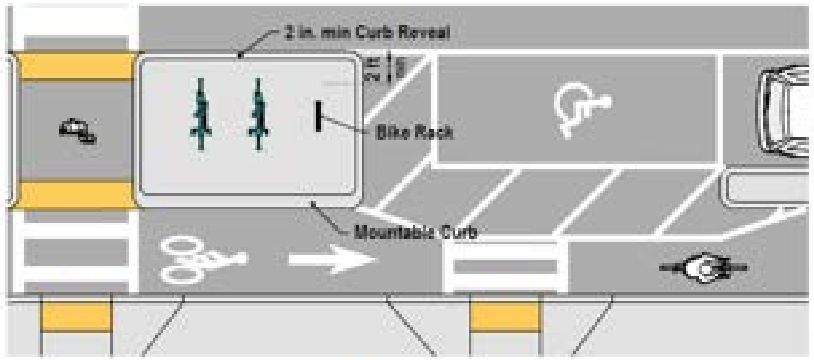
16.7 Bike Parking at Special Events

16.8 Bike Share Parking

16.9 Locker Rooms, Showers, and Repair Stations (End-of-Trip Facilities)

16.3. Short-Term Parking

16.3.4. Example Designs with Unique Considerations



Bike Parking on Raised Median Adjacent to Accessible Car Parking Space

Figure 16-2: On-Street Bicycle Corrals

Note:
Locking point of a single bicycle on the rack

Figure 16-6: Examples of Recommended and Not Recommended Racks

Table 16-1: Sample Short-Term Bicycle Parking Quantity Requirements

Short-Term Parking Quantities		
Types of Activity	Sample Bicycle Parking Quantities[a]	
	Most Contexts	Urbanized or High Bicycle Mode Share Areas
Multi-unit residential dwellings	0.05 spaces per bedroom	0.10 spaces per bedroom
Libraries and government buildings	One space per 10,000 square ft of floor area	One space per 8,000 square ft of floor area
Church, theatres, stadiums, parks, beaches	Spaces for 2 percent of maximum expected attendance	Spaces for 5 percent of maximum expected attendance
Schools (K-12)	One space per 20 students	1.5 spaces per 20 students
Colleges and universities	One space per 10 students of planned capacity	One space per 10 students of planned capacity
Rail or bus terminals and stations and airports	Spaces for 1.5 percent of a.m. peak passengers	Spaces for 2 percent of a.m. peak passengers
Retail- groceries	One space per 2,000 ft^2 of floor area	One space per 2,000 ft^2 of floor area
Retail- general	One space per 5,000 ft^2 of floor area	One space per 5,000 ft^2 of floor area
Office	One space per 20,000 ft^2 of floor area	One space per 20,000 ft^2 of floor area

[a] A minimum of two bike parking spots is recommended in all cases

Adapted from Anderson et al. (2010); see Chapter 16 References.

Thank you!
Questions?

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TOOLE DESIGN

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